— CHAPTER NUMBER

— DISCIPLINE

PLYWD

PREFAB

POS

F.A.

FB

FAB

FIRE ALARM

FACE BRICK

FLOOR DRAIN

FABRICATE

PLYWOOD

POSITIVE

PREFABRICATED

PANEL

—SS——— SANITARY LINE

TD———TD——— TELEPHONE & DATA

(IIII) C.B. —— CATCH BASIN

4885 \ REGISTEREC **ARCHITECT** JOSE O. BAZAN STATE OF WASHINGTON

BELADY GARAGE/ ACCESSORY BUILDING

7627 79TH AVE SE MERCER ISLAND, WASHINGTON

ARCHITECTS

2000 - 116TH AVENUE NE BELLEVUE, WA 98004

PHONE: 425.637.0831

PERMIT

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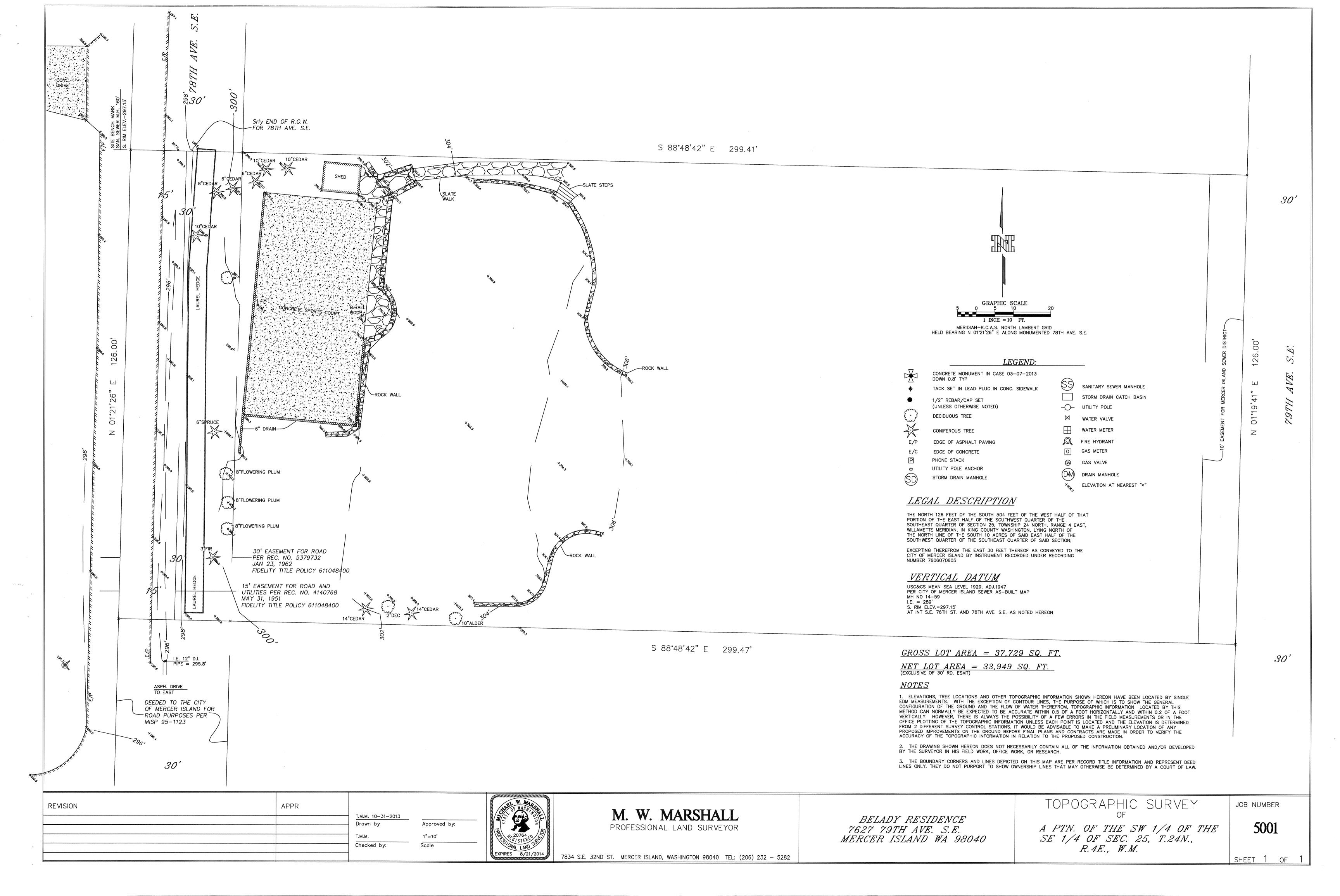
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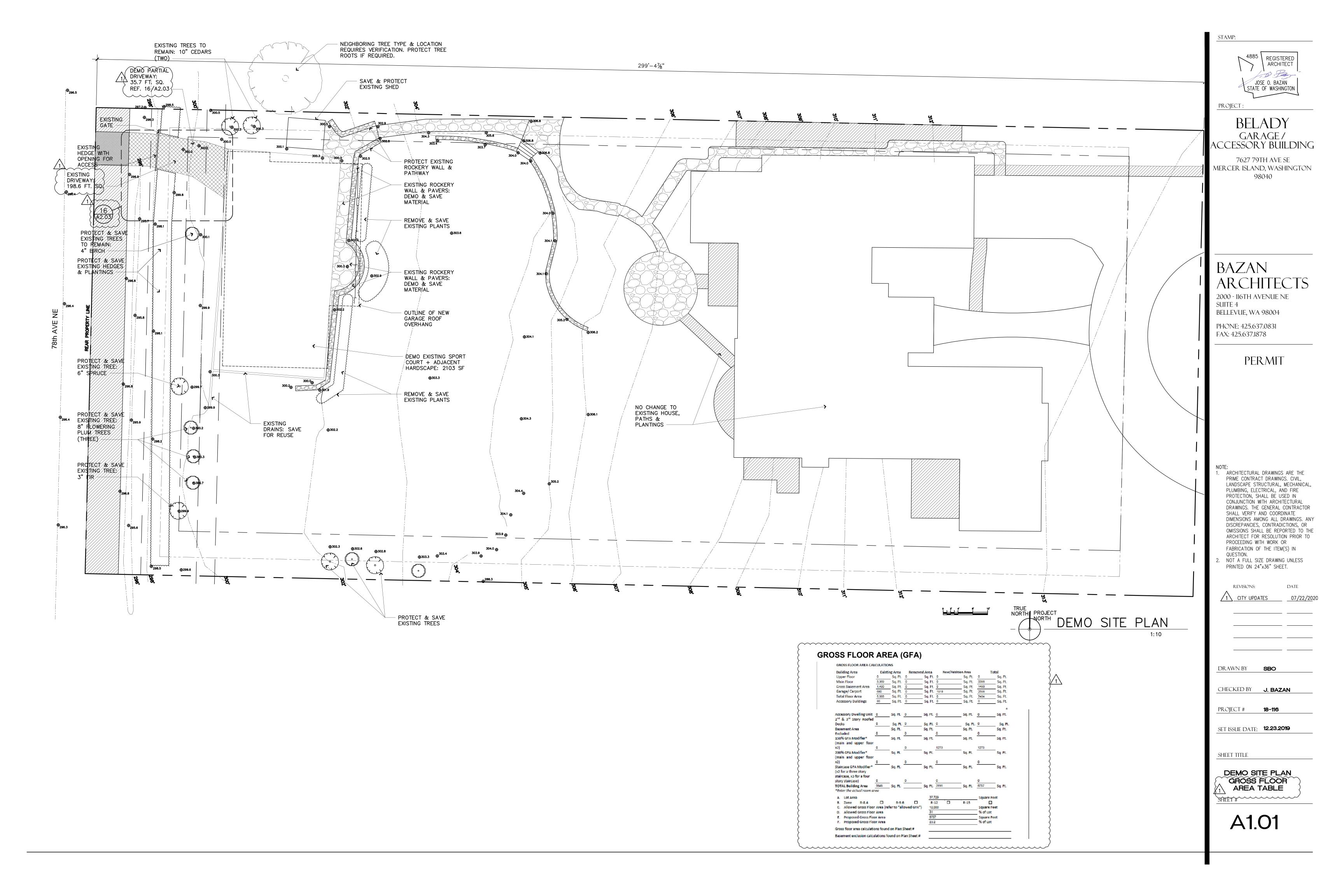
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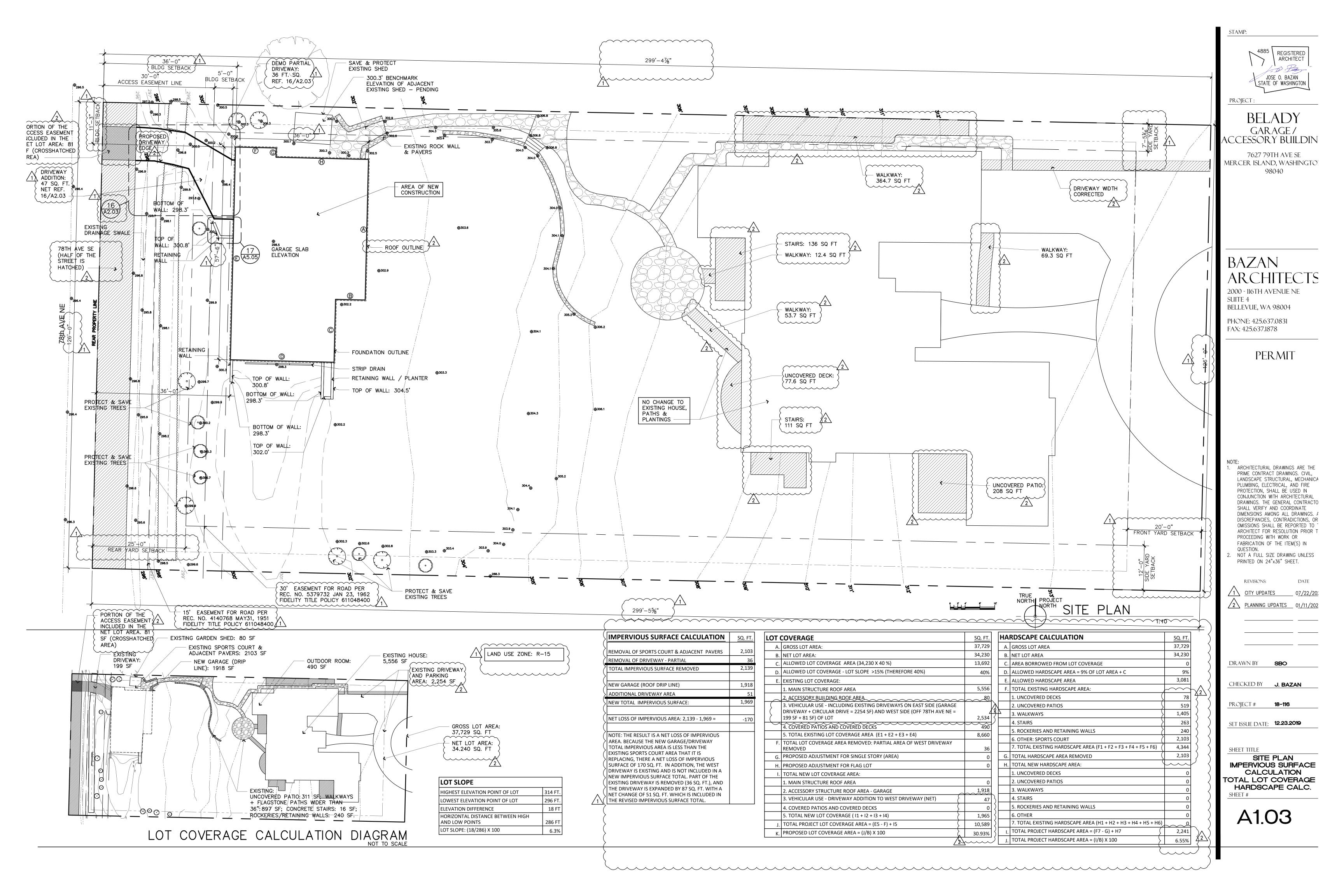
18-116

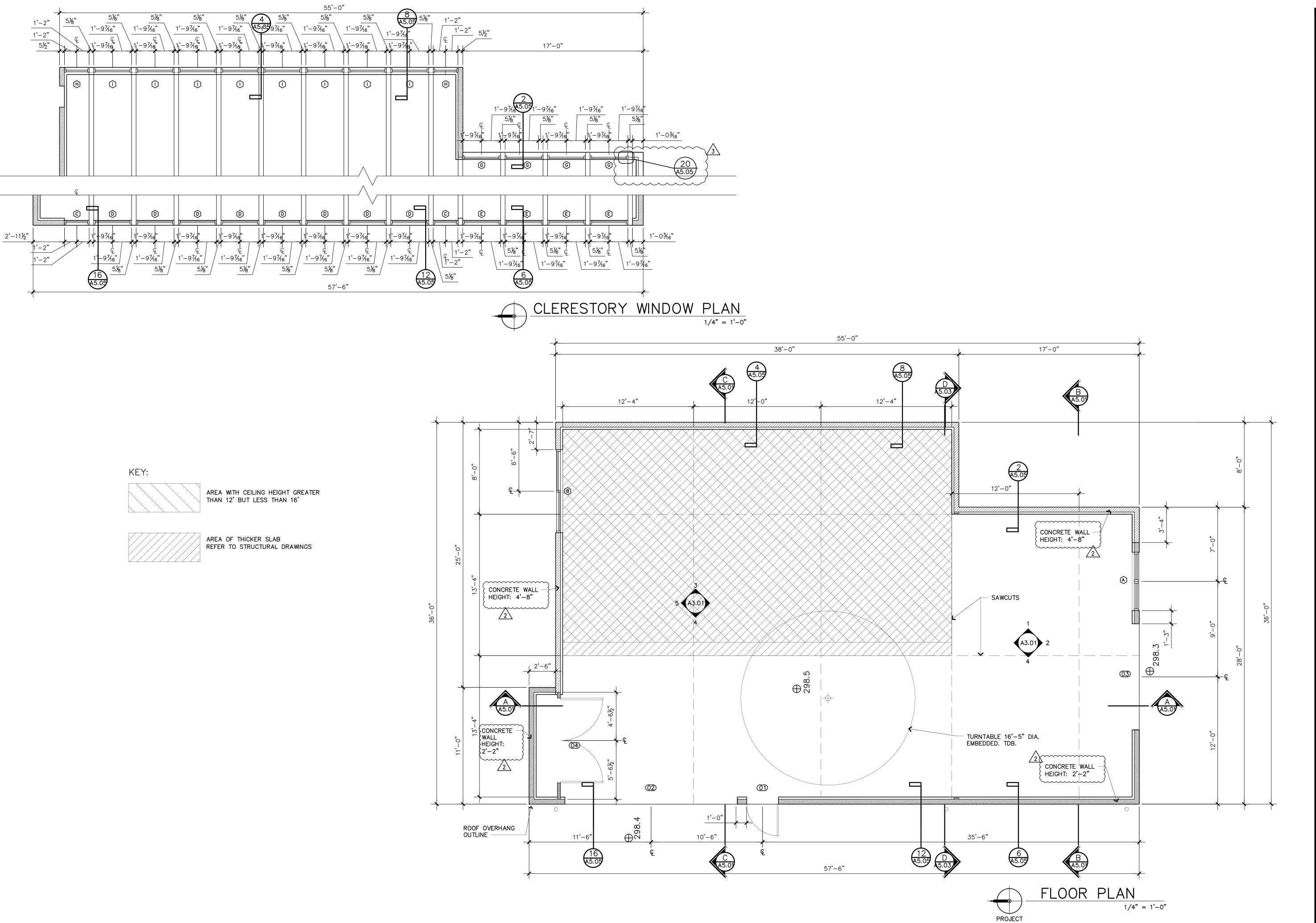
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JOSE O. BAZAN
STATE OF WASHINGTON

PROJECT:

BELADY GARAGE / ACCESSORY BUILDING

7627 79TH AVE SE MERCER ISLAND, WASHINGTON

BAZAN ARCHITECTS

2000 - 116TH AVENUE NE SUITE 4 BELLEVUE, WA 98004

PHONE: 425.637.0831 FAX: 425.637.1878

PERMIT

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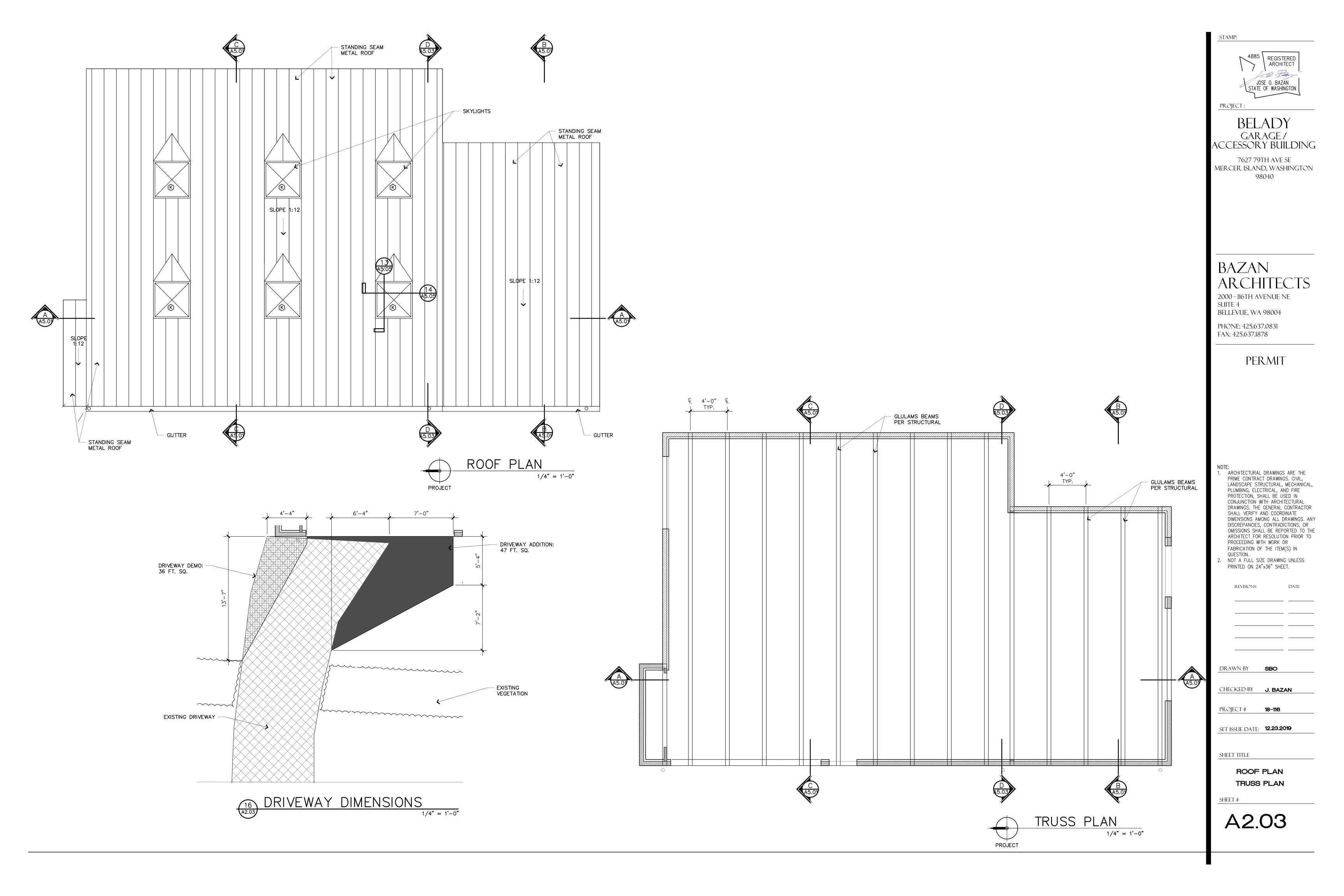
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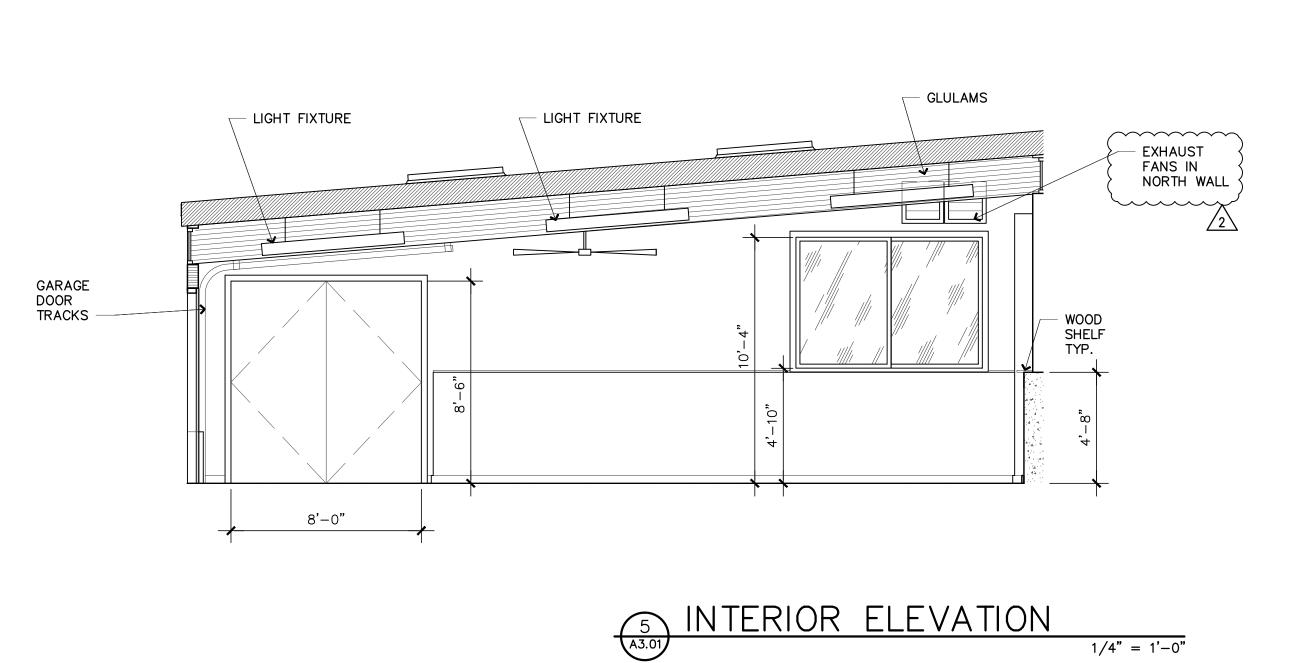
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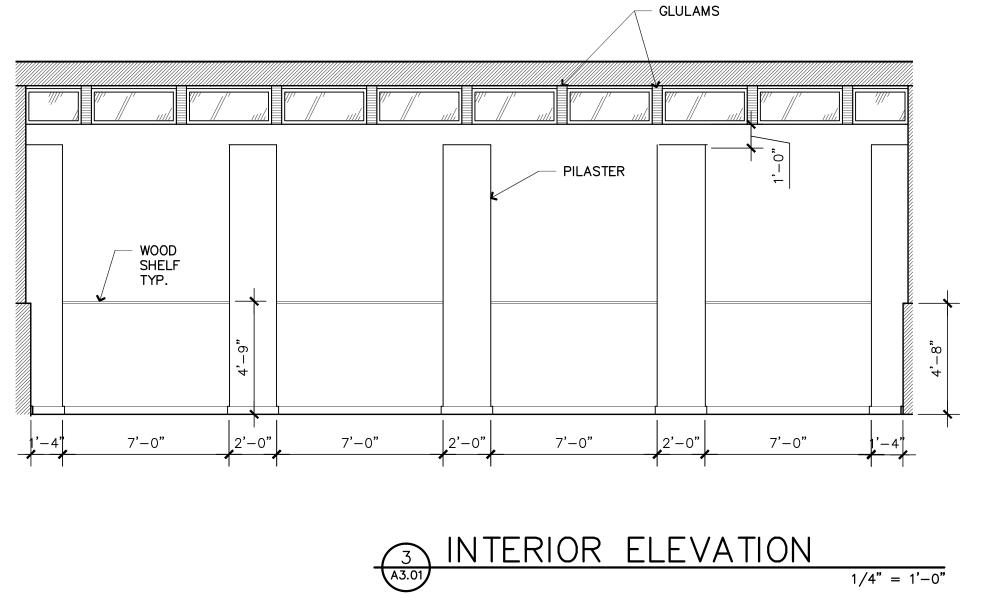
SHEET TITLE

FLOOR PLAN +
CLERESTORY
WINDOW PLAN

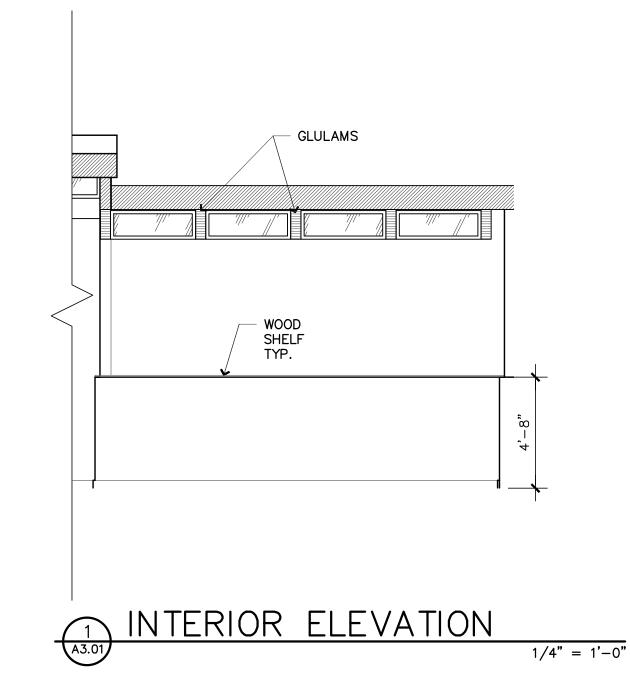
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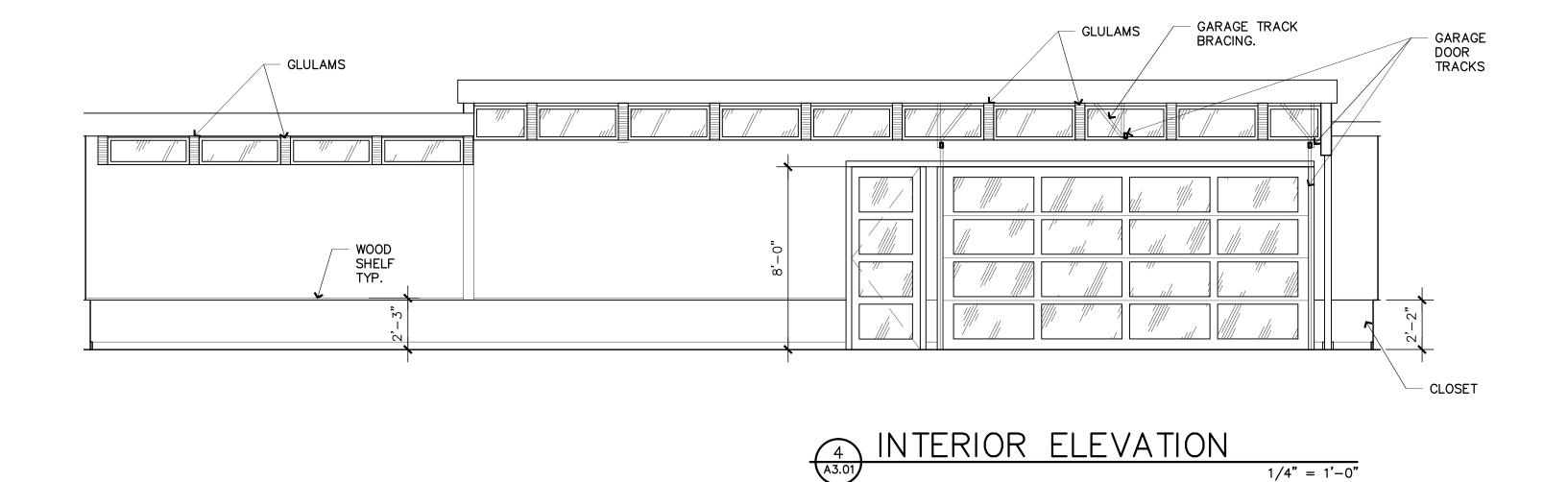




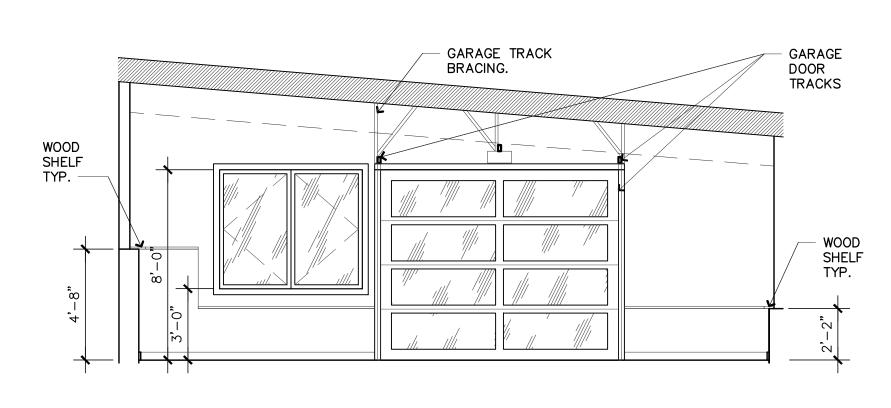


1/4" = 1'-0"





1/4" = 1'-0"



2 INTERIOR ELEVATION 1/4" = 1'-0"

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7627 79TH AVE SE MERCER ISLAND, WASHINGTON 98040

BAZAN ARCHITECTS

2000 - 116TH AVENUE NE SUITE 4 BELLEVUE, WA 98004

PHONE: 425.637.0831 FAX: 425.637.1878

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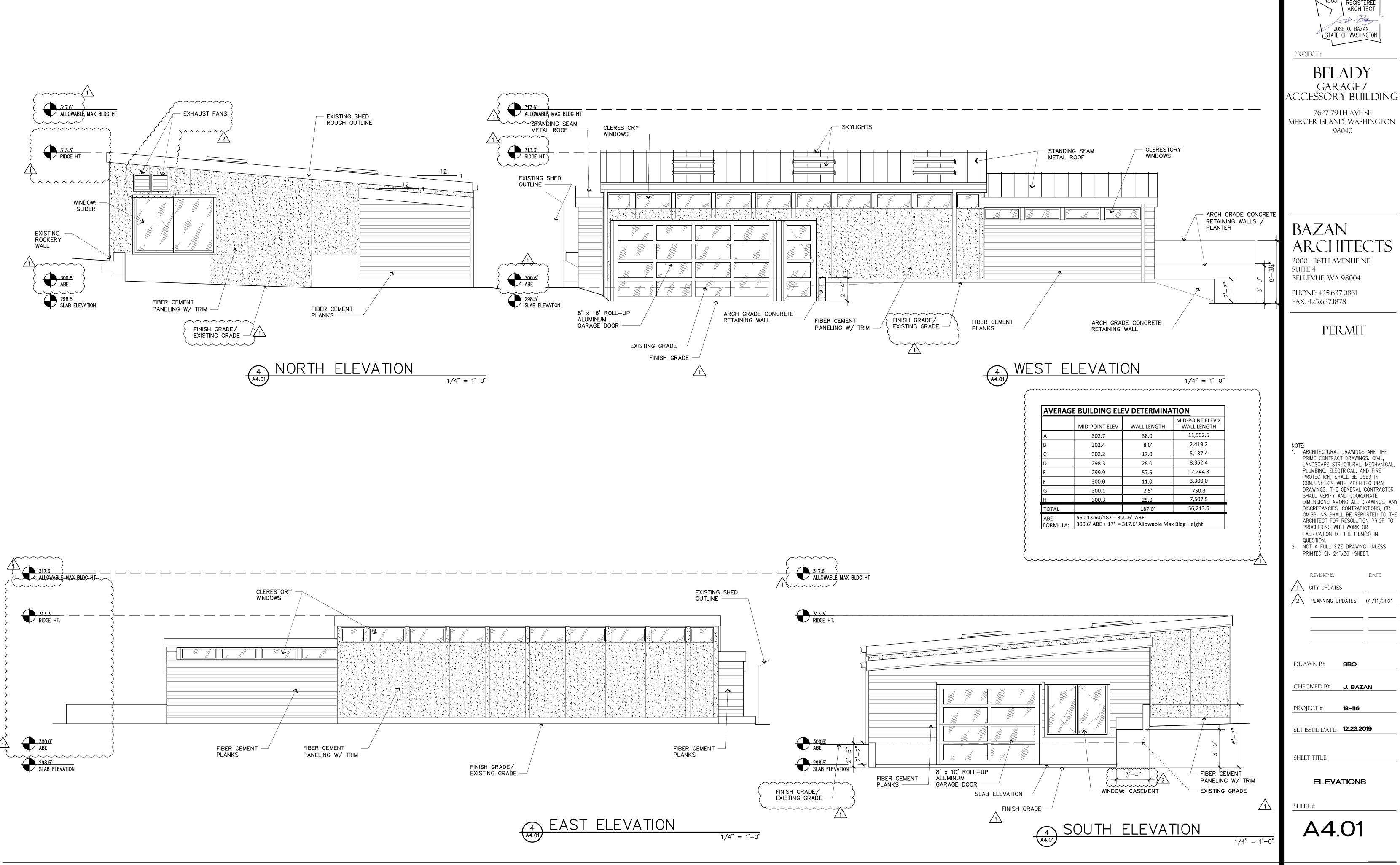
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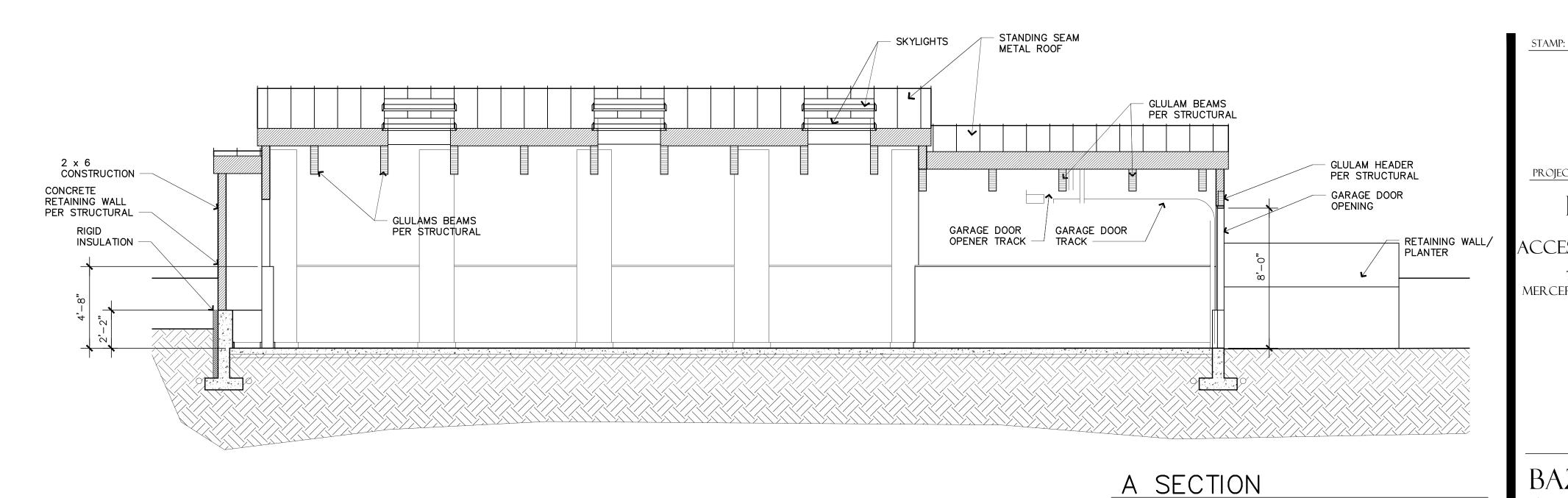
INTERIOR **ELEVATIONS**

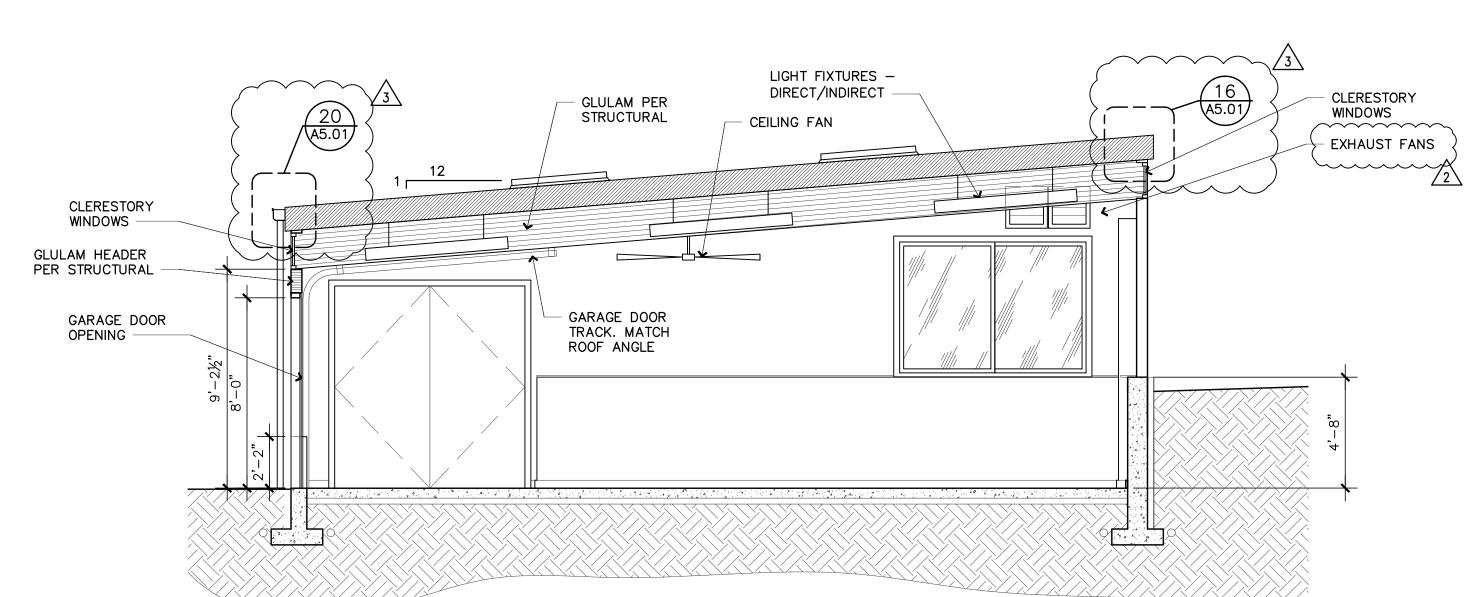
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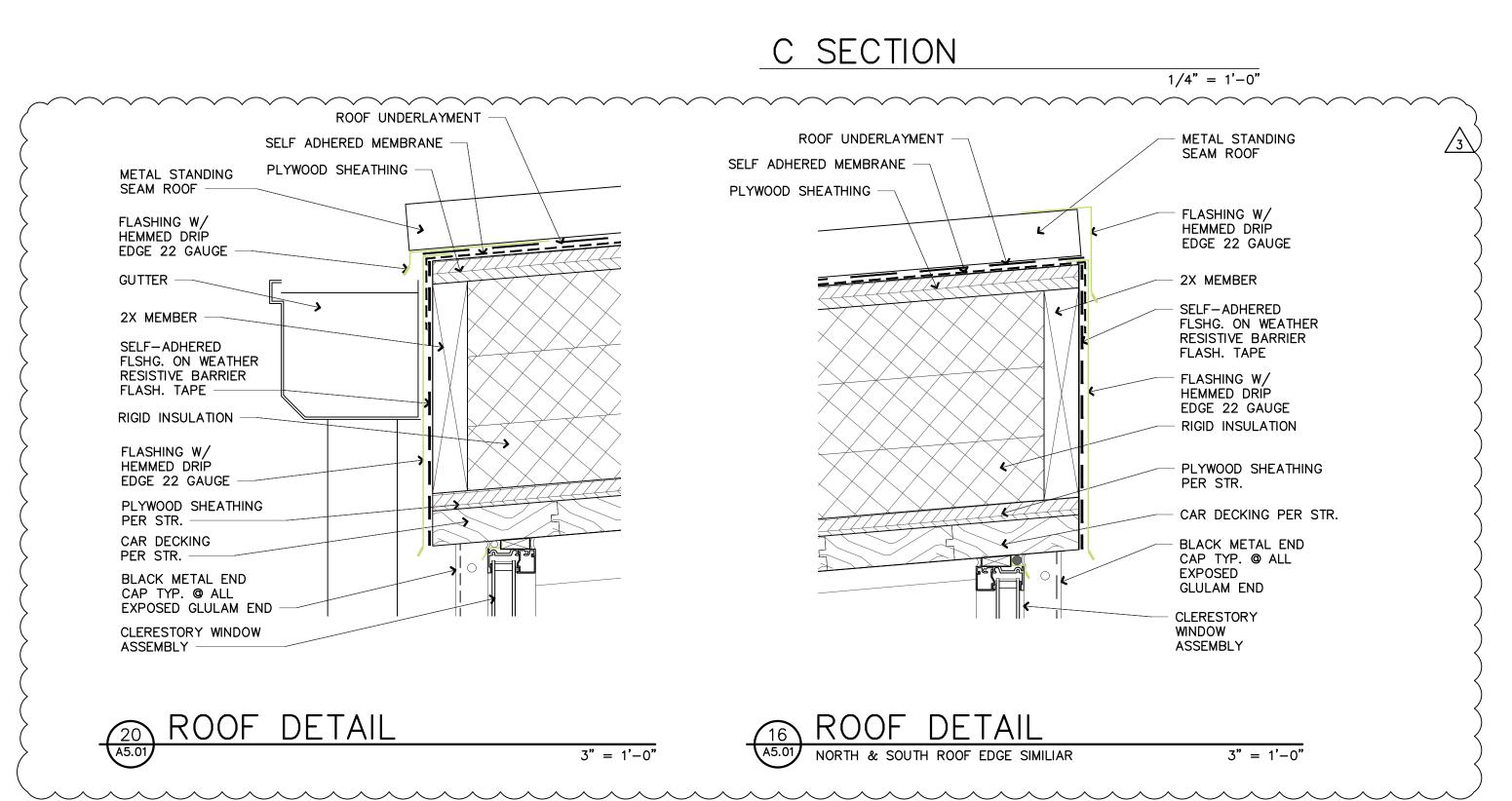


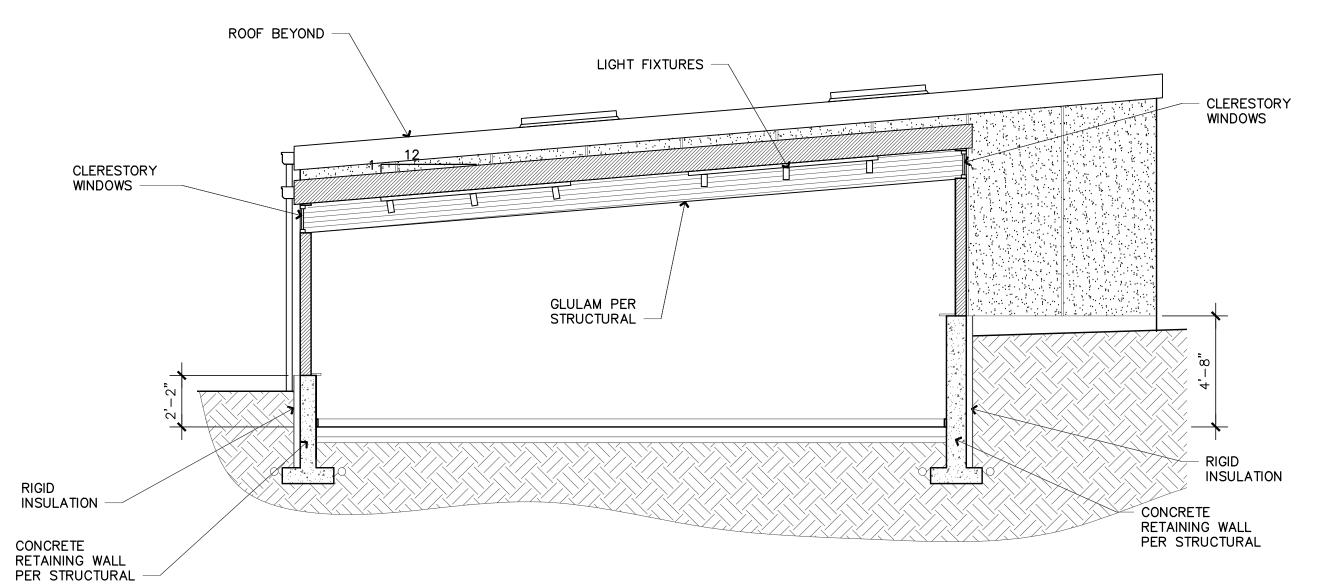
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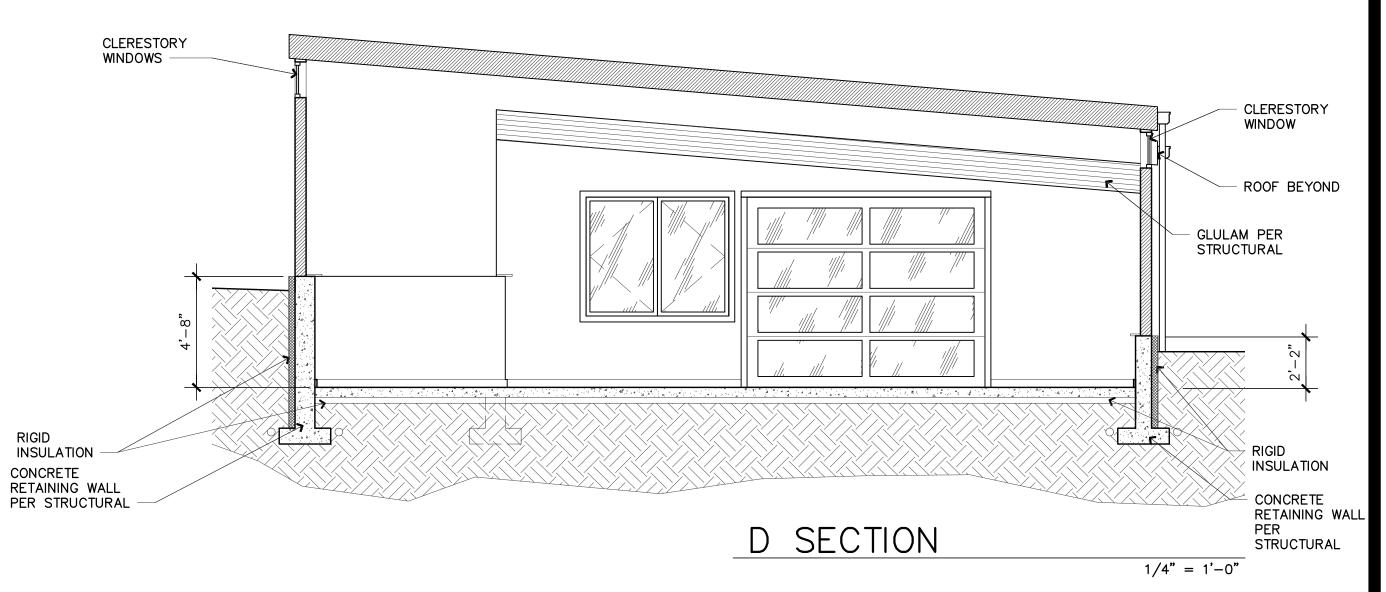
DIMENSIONS AMONG ALL DRAWINGS. ANY OMISSIONS SHALL BE REPORTED TO THE ARCHITECT FOR RESOLUTION PRIOR TO











B SECTION

4885 REGISTERED ARCHITECT

JOSE O. BAZAN STATE OF WASHINGTON

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7627 79TH AVE SE MERCER ISLAND, WASHINGTON 98040

BAZAN ARCHITECTS

2000 - 116TH AVENUE NE SUITE 4 BELLEVUE, WA 98004

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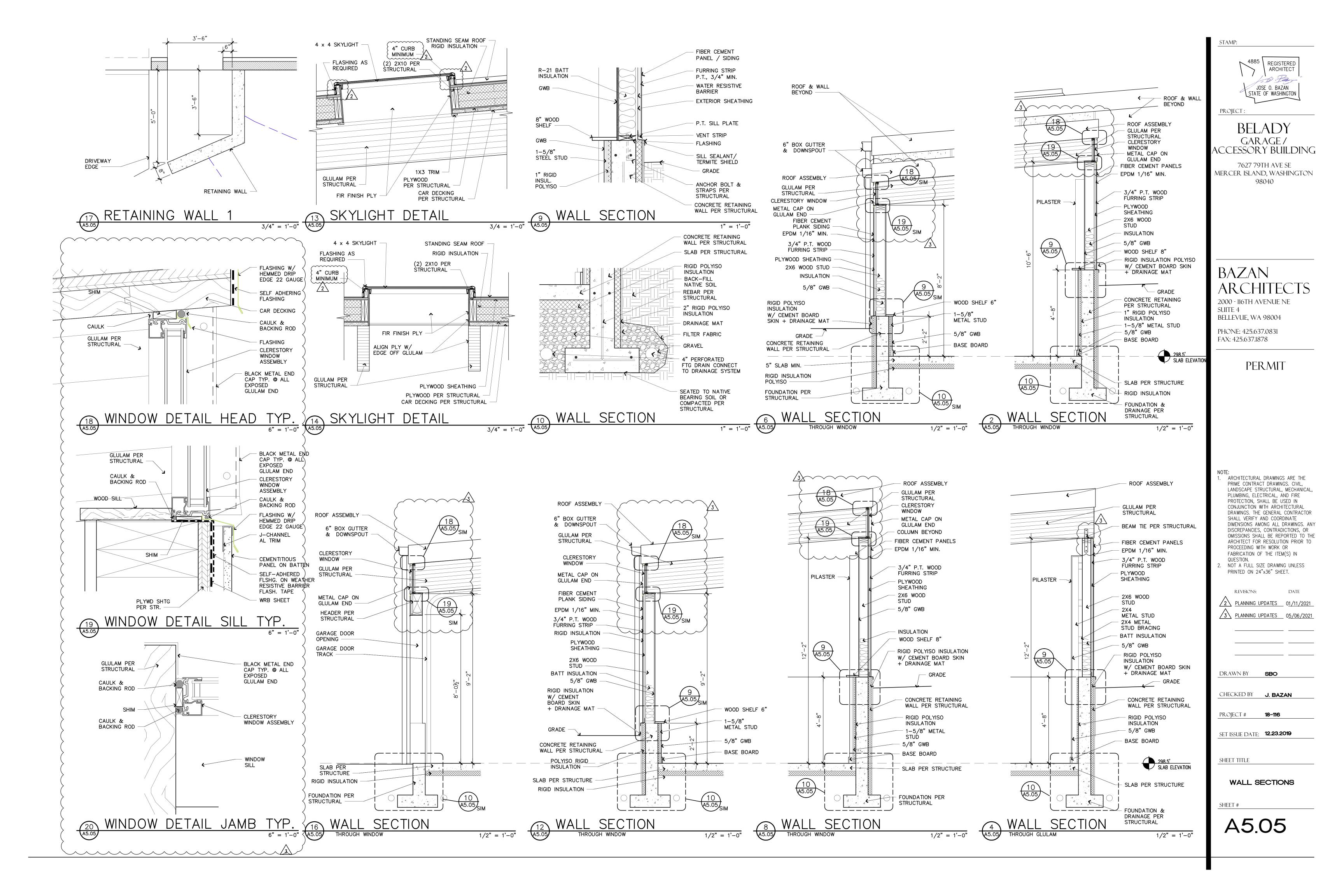
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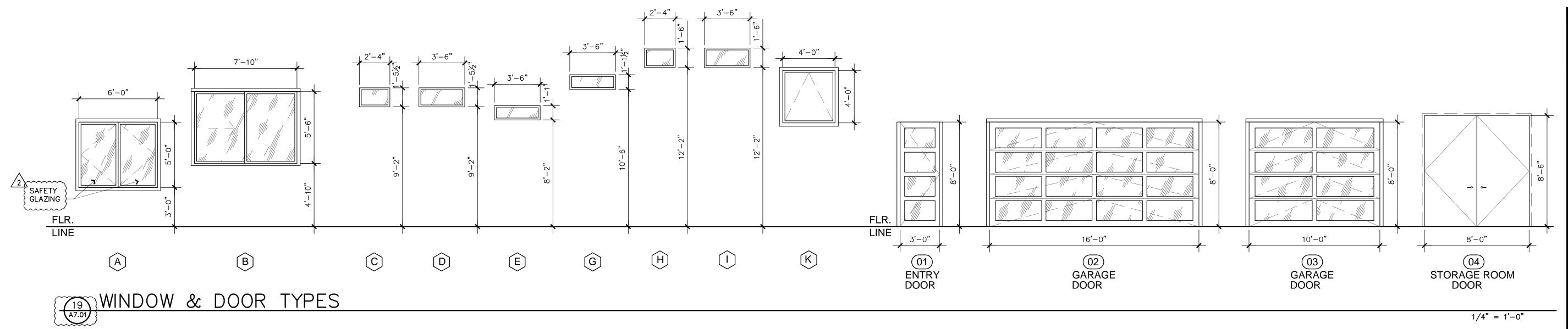
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SHEET TITLE
SECTIONS

SHEET #

A5.01





WINDOW SCHEDULE FIRE REQUIRE WINDOW SAFTY SQ. FT. REMARKS FRAME SHGC QUAN SYMBOL **EGRESS** WIDTH * HEIGHT * TYPE GLASS RATING U-VALUE MILGARD SERIES 710 YES /2 NO 6'-0" 5'-0″ CASEMENT ALUMINUM 150 / NR 24.9 MILGARD SERIES 710 7'-10" NO **ALUMINUM 150** NR 37.9 В NO 5'-6" SLIDER MILGARD SERIES 710 NO 2'-4" 1'-5 1/2" FIXED NO **ALUMINUM 150** 2.4 3'-6" 3.7 MILGARD SERIES 710 D NO 1'-5 1/2" FIXED NO **ALUMINUM 150** NR MILGARD SERIES 710 3'-6" 1'-1" NR 2.5 Ε NO FIXED NO **ALUMINUM 150** MILGARD SERIES 710 3'-6" 2.7 NO 1' 1 1/2" FIXED NO **ALUMINUM 150** MILGARD SERIES 710 2'-4" 1'-6″ FIXED **ALUMINUM 150** 2.5 MILGARD SERIES 710
VELUX 3'-6" 1'-6" **ALUMINUM 150** 3.9 4'-0" 4'-0" SKYLIGHT-OPERABLE WOOD CURB 13.4 NA NA * VERIFY ALL OPENINGS DOOR SCHEDULE SIZE: HEIGHT TYPE CORE FRAME SQ. FT. **ROOM NAME** SIZE: WIDTH MATERIAL U-VALUE SHGC QUAN REMARKS 3'-0" INSUL GLASS (1) TBD GARAGE 8'-0" 16'-0" GLASS (1) TBD GARAGE 8'-0" OVERHEAD GLASS (1) TBD 03 GARAGE 8'-0" 8'-0" OVERHEAD INSUL TBD 04 8'-0" 8'-6" STORAGE SWING WOOD WOOD (1) SAFETY NOTES: GLAZING

STAM

JOSE O. BAZAN STATE OF WASHINGTON

PROJECT

BELADY GARAGE / ACCESSORY BUILDING

7627 79TH AVE SE MERCER ISLAND, WASHINGTON

BAZAN 1" = 1'-ARCHITECTS

2000 - 116TH AVENUE NE SUITE 4 BELLEVUE, WA 98004

PHONE: 425.637.0831 FAX: 425.637.1878

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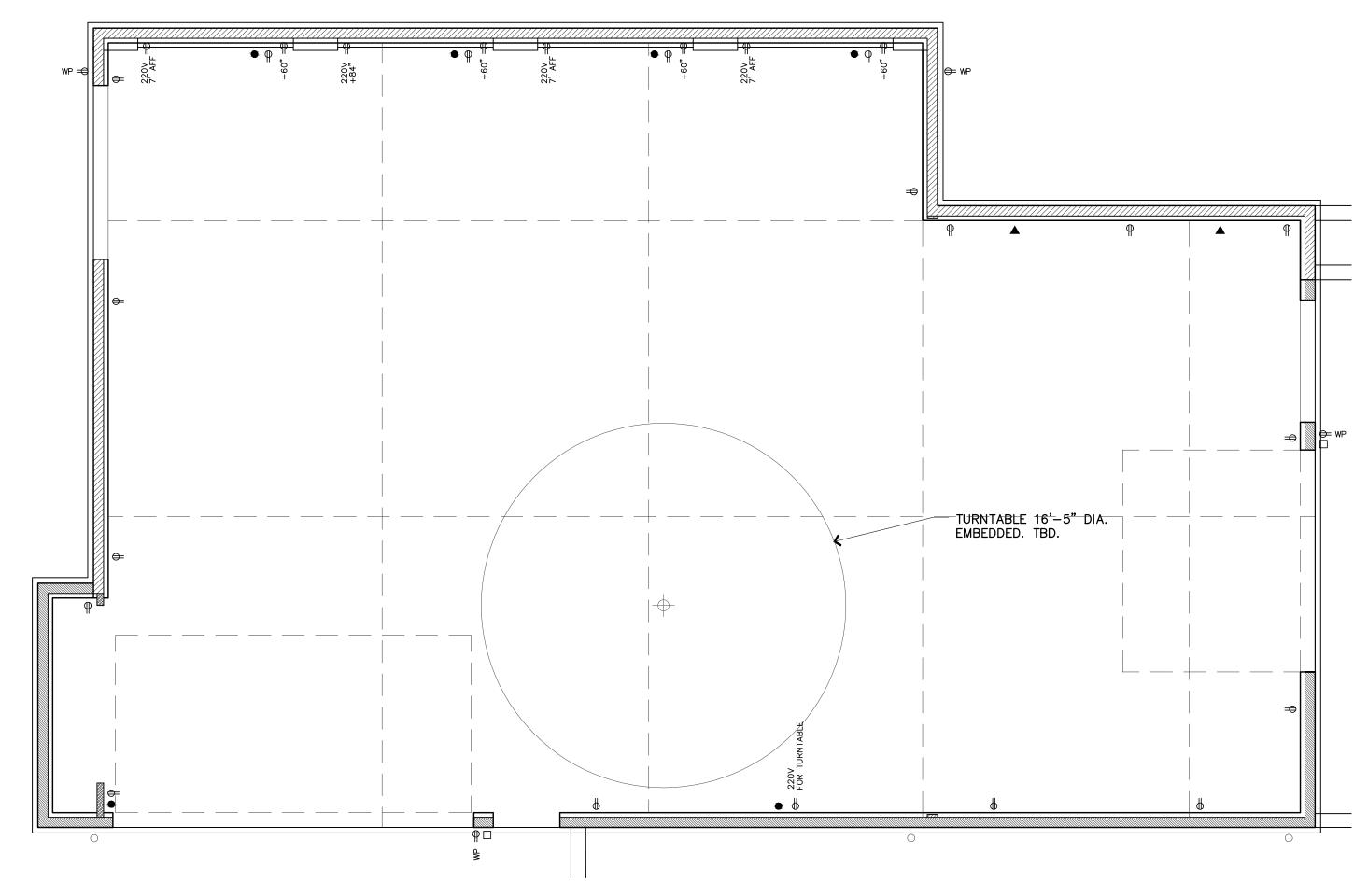
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SHEET TITLE

WINDOW + DOOR SCHEDULES

SHEET #

A7.01



TYPE	DESCRIPTION	LAMPS	WATTAGE	MANUFACTURER
	LINEAR LIGHTS SUSPENDED DOWN LED) TBD		
0 0 0	TRACK LIGHTING LED	TBD		
0	4" LED-WHITE	TBD		
Θ	4" LED-WHITE WALL WASHER	TBD		
	EXTERIOR WALL SCONCE	TBD		

LEGEND:

⊕
DUPLEX OUTLET @ 18" U.N.O.

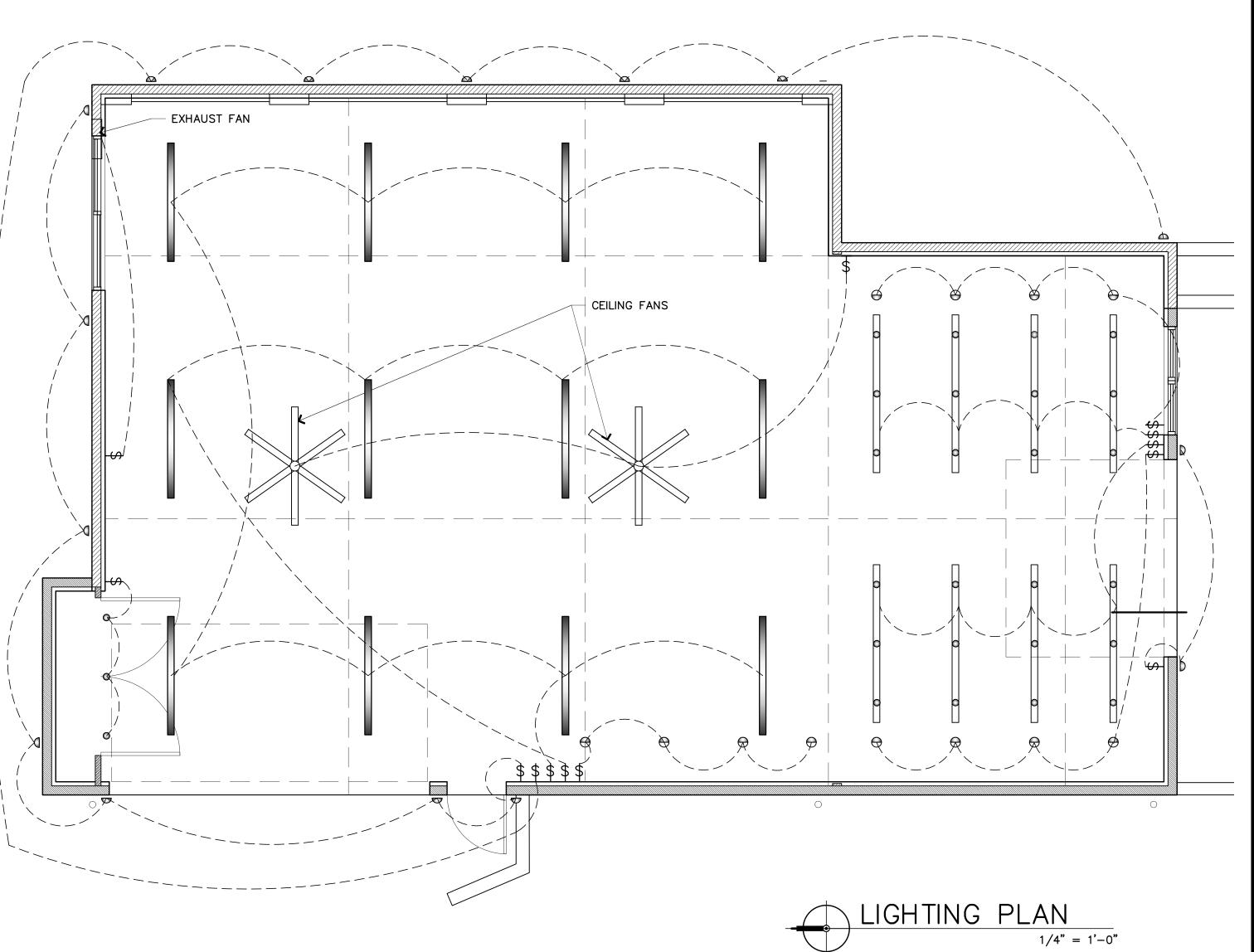
⊕ 220V
DUPLEX OUTLET 220V

⊕ + 60"
DUPLEX OUTLET AT DESIGNATED HEIGHT

⊕ WP
GROUND FAULT CIRCUT INTERRUPTER OUTLET W/WEATHER PROOF COVER

Image: Company of the company o





PROJECT

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SET ISSUE DATE: **12.23.2019**

SHEET TITLE

ELECTRICAL PLAN LIGHTING PLAN

SHEET #

E1.01

CODE: INTERNATIONAL BUILDING CODE (IBC)	2015
LOADINGS FLOOR LIVE LOAD DECK LIVE LOAD ROOF SNOW LOAD	40 PSF 60 PSF 25 PSF
WIND CRITERIA BUILDING CLASSIFICATIONULTIMATE WIND SPEEDWIND EXPOSURETOPOGRAPHIC FACTOR, Kzt	II 110 MPH B 1.6
SEISMIC CRITERIA SEISMIC RISK CATEGORYSPECTRAL RESPONSE COEFFICIENT, Ss SPECTRAL RESPONSE COEFFICIENT, S1 SEISMIC SITE CLASSSEISMIC DESIGN CATEGORY	II 1.47 0.56 D
STRUCTURAL SYSTEM	

ONE STORY WOOD FRAMED GARAGE

CONDITIONS BEFORE STARTING WORK.

GENERAL CONDITIONS

THE CONTRACTOR SHALL EXAMINE THE STRUCTURAL DRAWINGS AND SHALL NOTIFY THE STRUCTURAL ENGINEER IN WRITING OF ANY DISCREPANCIES HE MAY FIND BEFORE PROCEEDING THE WORK. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, ELEVATIONS AND SITE

ALL OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND THE STRUCTURAL ENGINEER BEFORE PROCEEDING WITH ANY WORK SO INVOLVED.

SPECIFIC NOTES AND DETAILS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. WHERE THE NOTES, DRAWINGS, AND/OR SPECIFICATIONS DIFFER, THE MORE STRINGENT REQUIREMENT SHALL APPLY.

4. IF A SPECIFIC DETAIL IS NOT SHOWN FOR ANY PART OF THE WORK, THE CONSTRUCTION SHALL BE THE SAME AS FOR SIMILAR WORK.

WORKING DIMENSIONS SHALL NOT BE SCALED FROM PLANS, SECTIONS, OR DETAILS ON THESE DRAWINGS.

THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ARCHITECT AND THE STRUCTURAL ENGINEER OF ANY CONDITION THAT, IN HIS OPINION, MIGHT ENDANGER THE STABILITY OF THE STRUCTURE OR CAUSE DISTRESS TO THE STRUCTURE.

THE CONTRACTOR SHALL SUPERVISE AND DIRECT HIS WORK AND HE SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES. PROVIDE ADEQUATE SHORING AND BRACING OF ALL STRUCTURAL MEMBERS DURING CONSTRUCTION. NOTIFY ENGINEER OF ALL FIELD CHANGES PRIOR TO INSTALLATION.

REFER TO THE ARCHITECTURAL DRAWINGS FOR INFORMATION NOT COVERED BY THESE GENERAL NOTES OR THE STRUCTURAL DRAWINGS.

ALL CONSTRUCTION SHALL BE DONE WITH MATERIALS, METHODS, AND WORKMANSHIP ACCEPTED AS GOOD PRACTICE BY THE CONSTRUCTION INDUSTRY AND IN CONFORMANCE WITH THE PROVISIONS OF PREVAILING CODE EDITION OF THE "INTERNATIONAL BUILDING CODE" (IBC) AND STANDARDS REFERENCED THEREIN.

10. PIPES, DUCTS, SLEEVES, OPENINGS, POCKETS, CHASES, BLOCK-OUTS, ETC., SHALL NOT BE PLACED IN SLABS, FOUNDATIONS, ETC., NOR SHALL ANY STRUCTURAL MEMBER BE CUT FOR SUCH ITEMS, UNLESS SPECIFICALLY DETAILED ON THESE STRUCTURAL DRAWINGS.

11. ALTERNATE ASSEMBLIES AND MATERIALS WILL BE CONSIDERED FOR REVIEW. ENGINEER MAY REQUEST PAYMENT FOR REVIEW.

FOUNDATION STRUCTURAL DESIGN COMPLIES WITH SOILS REPORT PRODUCED BY:

FOOTING BEARING PRESSURE:

1500 PSF (ASSUMED)

LATERAL EARTH PRESSURE ON RETAINING WALLS N.A.

SUBGRADE PREPARATION, DRAINAGE PROVISIONS, AND OTHER RELEVANT SOIL CONSIDERATIONS ARE TO BE IN ACCORDANCE WITH SAID SOILS REPORT.

DIMENSIONAL LUMBER, ANCHOR BOLT AND NAILING SPECIFICATIONS

1. MEET REQUIREMENTS OF PS 20-70 AND NATIONAL GRADING RULES FOR SOFTWOOD DIMENSIONAL LUMBER. BEAR STAMP OF WWPA.

2. MINIMUM DIMENSIONAL LUMBER GRADES TO BE:

WALL STUDS, 2X, 3 X...... HF STUD GRADE WALL PLATES, 2X, 3X....... HF STANDARD GRADE U.N.O JOISTS, 2 X 6:..... JOISTS, 2 X 8 AND UP...... DF #2 BEAMS, HEADERS, 6X DF #2

BEAMS, HEADERS, 4X...... DF #2, WWPA GRADING POSTS, 4X, 6X.... DF #2 U.N.O LUMBER NOT NOTED HERE... DF #2 U.N.O

3. PROVIDE STANDARD CUT WASHERS FOR BOLT HEADS AND NUTS BEARING AGAINST WOOD.

4. ALL SILLS OR PLATES RESTING ON CONCRETE OR MASONRY THAT IS IN CONTACT WITH OR RESTING ON FOUNDATIONS SHALL BE PRESSURE-TREATED DOUGLAS FIR/ HEMFIR IN ACCORDANCE TO WITH AWPA U1 (PLANT/SHOP TREATMENT) AND M4 (FIELD TREATMENT) STANDARDS. ALL BEARING WALL PLATES SHALL HAVE 5/8" Ø x10" J-BOLTS PLACED AT MAXIMUM OF 9" FROM THE END OF A PLATE AND SPACED AT INTERVALS SHOWN ON THE SHEARWALL SCHEDULE (MAXIMUM 4'-0" OC SPACING). PROVIDE BP PLATE WASHER AT ALL FOUNDATION SILL PLATE ANCHOR BOLTS. PROVIDE TWO ANCHOR BOLTS MINIMUM PER SECTION OF SILL. FOR NON-SHEARWALL, PLACE ANCHORS AT 48".

5. BOLTS IN WOOD SHALL NOT BE LESS THAN 7 DIAMETERS FROM THE END AND 4 DIAMETERS FROM THE EDGE OF THE MEMBER.

6. NAILS: COMMON WIRE NAILS. NAILING IN ACCORDANCE WITH IBC TABLE 2304.9.1.

7. PRESSURE TREATED WOOD: ALL NAILS INTO PT WOOD SHALL BE HOT DIPPED GALVANIZED PER ASTM A153 OR STAINLESS STEEL. ALL METAL CONNECTORS IN CONTACT WITH PT WOOD SHALL BE HOT DIPPED GALVANIZED AND MEET ASTM A653 CLASS G185 (1.85 OZ OF ZINC PER SQ FT MINIMUM) OR TYPE 304 / 316 STAINLESS STEEL SIMPSON Z-MAX CONNECTORS MEET THIS REQUIREMENT. FASTENERS AND CONNECTORS USED TOGETHER SHALL BE OF THE SAME TYPE (E.G. HOT DIPPED NAILS WITH HOT DIPPED HANGERS)

8. ALL LUMBER WITH A LEAST DIMENSION OF 2" (NOMINAL) SHALL BE STAMPED "SURFACE-DRY" AND SHALL HAVE A MOISTURE CONTENT WHEN SURFACED AND WHEN INSTALLED OF NO MORE THAN 19 PERCENT. LUMBER WITH A LEAST DIMENSION OF 4" (NOMINAL) OR GREATER SHALL BE STAMPED "SURFACE-GREEN" AND AIR-DRIED TO A MOISTURE CONTENT OF NOT MORE THAN 19 PERCENT PRIOR TO ITS USE IN FRAMING THE STRUCTURE.

9. NOTCHING AND BORING OF BEAMS AND JOISTS IS NOT ALLOWED WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD.

Y	N	1704.2.5 Inspection of Fabricators Verify fabrication/quality control procedures	Periodic			1705.4 Masonry Construction (A) Level A, B and C Quality Assurance:	
Y	N	1705.1.1 Special Cases (work unusual in nature, including but not limited to alternative materials		Υ	N	Verify compliance with approved submittals (B) Level B Quality Assurance:	Periodic
		and systems, unusual design applications, materials and systems with special manufacturer's		Υ	N	1. Verification of f m and f'AAC prior to construction	Periodic
		requirements)		Υ	N	(C) Level C Quality Assurance:1. Verification of f'm and f'AAC prior to construction and for every 5,000 SF during construction	Periodic
		1705.2 Steel Construction		Y	N	Verification of proportions of materials in premixed or preblended mortar, prestressing grout, and grout other than self-consolidating grout, as delivered to the project site	Continuous
Y	N	1. Fabricator and erector documents (Verify reports and certificates as listed in AISC 360, chapter N,	Each submittal	Υ	Ν	3. Verify placement of masonry units	Periodic
Y	N	paragraph 3.2 for compliance with construction documents) 2. Material verification of structural steel	Periodic	Υ	N	(D) Levels B and C Quality Assurance:1. Verification of Slump Flow and Visual Stability Index (VSI) of self-consolidating grout as delivered	Continuous
Y <	N N	3. Embedments (Verify diameter, grade, type, length, embedment. See 1705.3 for anchors) 4. Verify member locations, braces, stiffeners, and application of joint details at each connection	Continuous Periodic	V	N	to the project 2. Verify compliance with approved submittals	Periodic
•	14	comply with construction documents	1 chould	Ý	Ν	Verify proportions of site-mixed mortar, grout and prestressing grout for bonded tendons	Periodic
Y	N	 Structural steel welding: Inspection tasks Prior to Welding (Observe, or perform for each welded joint or member, the QA 	Observe or Perform as noted (4)	Y	N	Verify grade, type, and size of reinforcement and anchor bolts, and prestressing tendons and anchorages	Periodic
,	N	tasks listed in AISC 360, Table N5.4-1) b. Inspection tasks During Welding (Observe, or perform for each welded joint or member, the QA	Observe (4)	Y	N N	5. Verify construction of mortar joints6. Verify placement of reinforcement, connectors, and prestressing tendons and anchorages	Periodic Level B - Periodic
'		tasks listed in AISC 360, Table N5.4-1)	. ,	1			Level C - Continuous
Y	N	c. Inspection tasks After Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-3)	Observe or Perform as noted (4)	Y	N	7. Verify grout space prior to grouting	Level B - Periodic Level C - Continuous
,	NI	d. Nondestructive testing (NDT) of welded joints: see Commentary 1) Complete penetration groove welds 5/16" or greater in risk category III or IV	Periodic	Y	N N	Verify placement of grout and prestressing grout for bonded tendons Verify size and location of structural masonry elements	Continuous Periodic
Y	N	2) Complete penetration groove welds 5/16" or greater in risk category II	Periodic	Y	N	10. Verify type, size, and location of anchors, including details of anchorage of masonry to structural	Level B - Periodic
Y Y	N N	3) Thermally cut surfaces of access holes when material t > 2" 4) Welded joints subject to fatigue when required by AISC 360, Appendix 3, Table A-3.1	Periodic Periodic	Υ	N	members, frames, or other construction. 11. Verify welding of reinforcement (see 1705.2.2)	Level C - Continuous Continuous
Y	N	5) Fabricator's NDT reports when fabricator performs NDT 6. Structural steel bolting:	Each submittal (5)	Υ	N	12. Verify preparation, construction, and protestion of masonry during cold weather (temperature below 40oF) or hot weather (temperature above 90oF)	Periodic
Y	N	a. Inspection tasks Prior to Bolting (Observe, or perform tasks for each bolted connection, in	Observe or Perform as noted (4)	Υ	N	13. Verify application and measurement of prestressing force	Continuous
Y	N	accordance with QA tasks listed in AISC 360, Table N5.6-1) b.Inspection tasks During Bolting (Observe the QA tasks listed in AISC 360, Table N5.6-2)	Observe (4)	Y	N	 Verify placement of AAC masonry units and construction of thin-bed mortar joints (first 5000 SF of AAC masonry) 	Continuous
		1) Pre-tensioned and slip-critical joints		Υ	N	15. Verify placement of AAC masonry units and construction of thin-bed mortar joints (after the first	Level B - Periodic
Y Y	N N	a) Turn-of-nut with matching markings b) Direct tension indicator	Periodic Periodic	Υ	N	5000 SF of AAC masonry) 16. Verify properties of thin-bed mortar for AAC masonry (first 5000 SF of AAC masonry)	Level C - Continuous Continuous
Y Y	N N	c) Twist-off type tension control bolt d) Turn-of-nut without matching markings	Periodic Continuous	Y	N	17. Verify properties of thin-bed mortar forAAC masonry (after the first 5000 SF of AAC masonry)	Level B - Periodic Level C - Continuous
Ϋ́	N	e) Calibrated wrench	Continuous	Υ	N	18. Prepare grout and mortar specimens	Level B - Periodic
Y	N	Snug-tight joints Inspection tasks After Bolting (Perform tasks for each bolted connection in accordance with QA	Periodic Perform (4)	Υ	N	19. Observe preparation of prisms	Level C - Continuous Level B - Periodic
	NI.	tasks listed in AISC 360, Table N5.6-3)	• •	•			Level C - Continuous
Y	N	7. Inspection of steel elements of composite construction prior to concrete placement in accordance with QA tasks listed in AISC 360, Table N6.1	Observe or Perform as noted (4)	Υ	N	1705.5 Wood Construction1. Inspection of the fabrication process of wood structural elements and assemblies in accordance	Periodic
		1705.2.2 Steel Construction Other Than Structural Steel		V	N	with Section 1704.2.5 2. For high-load diaphragms, verify grade and thickness of structural panel sheathing agree with	Periodic
		Material verification of cold-formed steel deck:		I		approved building plans	
Y Y	N N	a. Identification markings b. Manufacturer's certified test reports	Periodic Each submittal	Y	N	For high-load diaphragms, verify nominal size of framing members at adjoining panel edges, nail or staple diameter and length, number of fastener lines, and that spacing between fasteners in each	Periodic
,		2. Connection of cold-formed steel deck to supporting structure:		.,		line and at edge margins agree with approved building plans	-
Y Y	N N	a. Welding b. Other fasteners (in accordance with AISC 360,Section N6)	Periodic	Y	N	4. Metal-plate-connected wood trusses spanning 60 feet or greater: verify temporary and permanent restraint/bracing are installed in accordance with the approved truss submittal package	Periodic
Y	N N	Verify fasteners are in conformance with approved submittal Verify fastener installation is in conformance with approved submittal and manufacturer's	Periodic Periodic			1705.6 Soils	
ī	IN	recommendations	Periodic			1. Verify materials below shallow foundations are adequate to achieve the design bearing capacity.	
Y	N	Reinforcing steel A. Verification of weldability of steel other than ASTM A706	Periodic	Y Y	N N	 Verify excavations are extended to proper depth and have reached proper material. Perform classification and testing of controlled fill materials. 	Periodic Periodic
Y	N	b. Reinforcing steel resisting flexural and axial forces in intermediate and special moment frames,	Continuous	Ϋ́	N	4. Verify use of proper materials, densities, and lift thicknesses during placement and compaction of	Periodic
Y	N	boundary elements of special concrete structural walls and shear reinforcement c. Shear reinforcement	Continuous	Y	N	controlled fill 5. Prior to placement of controlled fill, observe subgrade and verify that site has been prepared	Continuous
Y	N	d. Other reinforcing steel 4. Cold-formed steel trusses spanning 60 feet or greater	Periodic	Υ	N	properly	Periodic
Y	N	a. Verify temporary and permanent restraint/bracing are installed in accordance with the approved	Periodic			1705.7 Driven Deep Foundations	
		truss submittal package		Y	N N	 Verify element materials, sizes and lengths comply with requirements Determine capacities of test elements and conduct additional load tests, as required 	Continuous Continuous
		1705.3 Concrete Construction		Ý	N	3. Observe driving operations and maintain complete and accurate records for each element	Continuous
Y Y	N N	Inspection of reinforcing steel installation (see 1705.2.2 for welding) Inspection of prestressing steel installation	Periodic. Periodic	Y	N	4. Verify placement locations and plumbness, confirm type and size of hammer, record number of blows per foot of penetration, determine required penetrations to achieve design capacity, record tip	Continuous
Ý	N	3. Inspection of anchors cast in concrete where allowable loads have been increased per section	Continuous	V		and butt elevations and document any damage to foundation element	0 0
Y	N	1908.5 or where strength design is used 4. Inspection of anchors and reinforcing steel post-installed in hardened concrete: Per research	Periodic or as required by the research report issued by an	Ϋ́Υ	N N	5. For steel elements, perform additional inspections per Section 1705.26. For concrete elements and concrete-filled elements, perform additional inspections per Section	See Section 1705.2 See Section 1705.3
		reports including verification of anchor type, anchor dimensions, hole dimensions, hole cleaning procedures, anchor spacing, edge distances, concrete minimum thickness, anchor embedment	approved source	v	N	1705.3 7. For specialty elements, perform additional inspections as determined by the registered design	In accordance with construction documents
		and tightening torque		ı		professional in responsible charge	
Y Y	N N	5. Verify use of approved design mix6. Fresh concrete sampling, perform slump and air content tests and determine temperature of	Periodic Continuous	Y	N	8. Perform additional inspections and tests in accordance with the construction documents	In accordance with construction documents
		concrete		V		1705.8 Cast-in-Place Deep Foundations	Oasting
Y Y	N N	7. Inspection of concrete and shotcrete placement for proper application techniques8. Inspection for maintenance of specified curing temperature and techniques	Continuous Periodic	Y Y	N N	1.Observe drilling operations and maintain complete and accurate records for each element 2. Verify placement locations and plumbness, confirm element diameters, bell diameters (if	Continuous Continuous
,	N	Inspection of prestressed concrete: Application of prestressing force	Continuous			applicable), lengths, embedment into bedrock (if applicable) and adequate end-bearing strata capacity. Record concrete or grout volumes	
r Y	N	b. Grouting of bonded prestressing tendons in the seismic-force-resisting system	Continuous	Υ	N	3. For concrete elements, perform additional inspections in accordance with Section 1705.3	See Section 1705.3
Y	N	Erection of precast concrete members Inspect in accordance with construction documents	In accordance with construction documents	Y	N	4. Perform additional inspections and tests in accordance with the construction documents	In accordance with construction documents
Y	N	b. Perform inspections of welding and bolting in accordance with Section 1705.2	In accordance with Section 1705.2			1705.9 Helical Pile Foundations	0 "
Y	N	11. Verification of in-situ concrete strength, prior to stressing of tendons in post tensioned concrete and prior to removal of shores and forms from beams and structural slabs	Periodic	Y	N	 Verify installation equipment, pile dimensions, tip elevations, final depth, final installation torque and other data as required. 	Continuous
Y V	N N	12. Inspection of formwork for shape, lines, location and dimensions 13. Concrete strength testing and verification of compliance with construction documents	Periodic Periodic	Y	N	2. Perform additional inspections and tests in accordance with the construction documents	In accordance with construction documents
•	14	13. Somercic strength testing and verification of compliance with construction documents	1 chodic			1705.10.1 Structural Wood Special Inspections For Wind Resistance	
		Notes:		Y Y	N N	 Inspection of field gluing operations of elements of the main windforce-resisting system Inspection of nailing, bolting, anchoring and other fastening of components within the main 	Continuous Periodic
		1. The inspection and testing agent(s) shall be engaged by the Owner or the Owner's Agent, and not			.,	windforce-resisting system	, oneae
		by the Contractor or Subcontractor whose work is to be inspected or tested. Any conflict of interest must be disclosed to the Building Official prior to commencing work. The qualifications of the				1705.10.2 Cold-formed Steel Special Inspections For Wind Resistance	
		Special Inspector(s) and/or testing agencies may be subject to the approval of the Building Official and/or the Design Professional.		Y	N N	 Inspection during welding operations of elements of the main windforce-resisting system Inspections for screw attachment, bolting, anchoring and other fastening of components within the 	Periodic Periodic
		The list of Special Inspectors may be submitted as a separate document, if noted so above.		ı	IN	main windforce-resisting system	T CHOULE
						1705.10.3 Wind-resisting Components	
		3. Special Insepctions as required by Section 1704.2.5 are not required where the fabricator is approved in accordance with IBC Section 1704.2.5.2		Y	N N	1. Roof cladding 2. Wall cladding	Periodic Periodic
				ı			
		4. Observe on a random basis, operations need not be delayed pending these inspections. Perform these tasks for each welded joint, bolted connection, or steel element.		Y	N	1705.11.1 Structural Steel Special Inspections for Seismic Resistance Inspection of structural steel in accordance with AISC 341	In accordance with AISC 341
		5. NDT of welds completed in an approved fabricator's shop may be performed by that fabricator when approved by the AHJ. Refer to AISC 360, N7.		Υ	N	1705.11.2 Structural Wood Special Inspections for Seismic Resistance 1. Inspection of field gluing operations of elements of the seismic-force resisting system	Continuous
				Υ	N	Inspection of nailing, bolting, anchoring and other fastening of components within the seismic-force-resisting system	Periodic
						• •	
						1705.11.3 Cold-formed Steel Light-Frame Construction Special Inspections for Seismic Resistance	
CRET	E AND RE	INFORCING		Y	N	Inspection during welding operations of elements of the seismic-force-resisting system	Periodic Periodic
CC	ONCRETE	SHALL CONFORM TO THE INDICATED REFERENCE CODES AND STANDARDS		Y	N	Inspections for screw attachment, bolting, anchoring and other fastening of components within the seismic-force-resisting system	Periodic
		MODIFIED BELOW:					
ΑC	CI-301 - "S	TANDARD SPECIFICATIONS FOR STRUCTURAL CONCRETE"	STRUCTURAL AND MISCELLANEOUS STEEL				
		UILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE"	STEEL MEMBERS HARDWARE FASTENERS SHALL BE H		CALVA	NIZED OD EDOVY DAINTED DED ADCHITECT	

REQUIRED? (Y/N) MATERIAL / ACTIVITY

EXTENT

Periodic

CONCRE

REQUIRED? (Y/N) MATERIAL / ACTIVITY

ACI-318 - "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" ACI-305R - "HOT WEATHER CONCRETING" ACI-306R - "COLD WEATHER CONCRETING" ACI-304 - "GUIDE FOR MEASURING, MIXING, TRANSPORTING AND PLACING CONCRETE"

CONCRETE MIX SPECIFICATIONS

LOCATION COMP. SRENGTH W/C RATIO AIR CONTENT REMARK FOOTING 2500 PSI (MIN. OF 5.5 SACKS OF CEMENT PER CUBIC YARD OF CONCRETE) SLAB ON GRADE 2500 PSI (MIN. OF 5.5 SACKS OF CEMENT PER CUBIC YARD OF CONCRETE) FOUNDATION WALL 2500 PSI (MIN. OF 5.5 SACKS OF CEMENT PER CUBIC YARD OF CONCRETE) **TOPPING**

TOTAL AIR CONTENT IS SPECIFIED IN THE TABLE ABOVE. AIR CONTENT TOLERANCE SHALL BE ± 1% AND SHALL BE MEASURED AT THE POINT OF PLACEMENT. (AFTER PUMPING IF APPLICABLE). ALL CONCRETE EXPOSED TO THE WEATHER SHALL HAVE AN APPROVED ADMIXTURE TO ENTRAIN AIR - 5% TOTAL AIR REQUIRED. CONCRETE THAT CAN BE SUBJECTED TO FREEZING AND THAWING DURING CONSTRUCTION SHALL BE AIR ENTRAINED.

3. PROVIDE GRADE 60 KSI (A615) FOR CONCRETE STEEL REINFORCING

STEEL MEMBERS, HARDWARE, FASTENERS SHALL BE HOT DIPPED GALVANIZED OR EPOXY PAINTED PER ARCHITECT REQUIREMENTS. ALL CUT, REPAIRED AND EXPOSED SURFACE SHALL BE PAINTED WITH (2) COAT OF 95% ZINC RICH PAINT PER ASTM A780. COLOR TO MATCH EXISTING.

STEEL SHALL CONFORM TO THE FOLLOWING STANDARDS:

TUBE COLUMNS: ASTM A500, GRADE B (Fy = 46 KSI) WIDE FLANGE COLUMNS / BEAWASTM 572 GR50 SCHEDULE 40, CONFORMING TO ASTM A53, TYPE E OR S, GRADE B (Fy = 35 KSI.) STEEL PIPE:

ALL OTHER STEEL: ASTM A36 (Fy = 36 KSI) OR ASTM A992 ASTM A307 (WOOD/STEEL CONN) BOLTS: ASTM A325/A490 WITH LOCK WASHERS (STEEL/STEEL AND STEEL/CONC CONN) BOLTS: ANCHOR BOLTS: ASTM A307 (WOOD FRAMING) ASTM A325 (STEEL FRAMING) ANCHOR BOLTS:

ALL SLIP CRITICAL CONNECTIONS SHALL BE ASTM A325 BOLTS AND SHALL BE ENGINEER-APPROVED, SELF-LOAD INDICATING TYPES, AND SHALL BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.

CONFORM TO THE AWS CODES D1.1 AND D1.3, AND USE ONLY CERTIFIED WELDERS. WELDS NOT SPECIFIED ARE TO BE 1/4" CONTINUOUS FILLET MINIMUM. INCREASE WELD SIZE TO AWS MINIMUM SIZES, BASED ON PLATE THICKNESS. USE DRY E70 ELECTRODES. ALL WELDING SHALL CONFORM TO THE AWS CODES, AND SHALL BE BY CERTIFIED WELDERS. WELDS NOT SPECIFIED SHALL BE 1/4" CONTINUOUS FILLET MINIMUM. USE DRY E70 ELECTRODES.

	DRAWING LIST	
SHEET NUMBER	SHEET NAME	ISSUE DATE
S-0	GENERAL NOTES AND SPECIFICATIONS	05-04-21
S-1	FRAMING PLAN	05-04-21
S-2	FRAMING PLAN	05-04-21

05-04-21

05-04-21

S-3

Grand total: 5

FRAMING DETAILS

FRAMING DETAILS

EXTENT

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BELADY GARAGE

7627 79TH AVE SE, MERCER ISLAND, WA

DRAWING INFO

ISSUE DATE 05-04-21

ISSUED FOR PERMIT

PROJECT NO. 19207

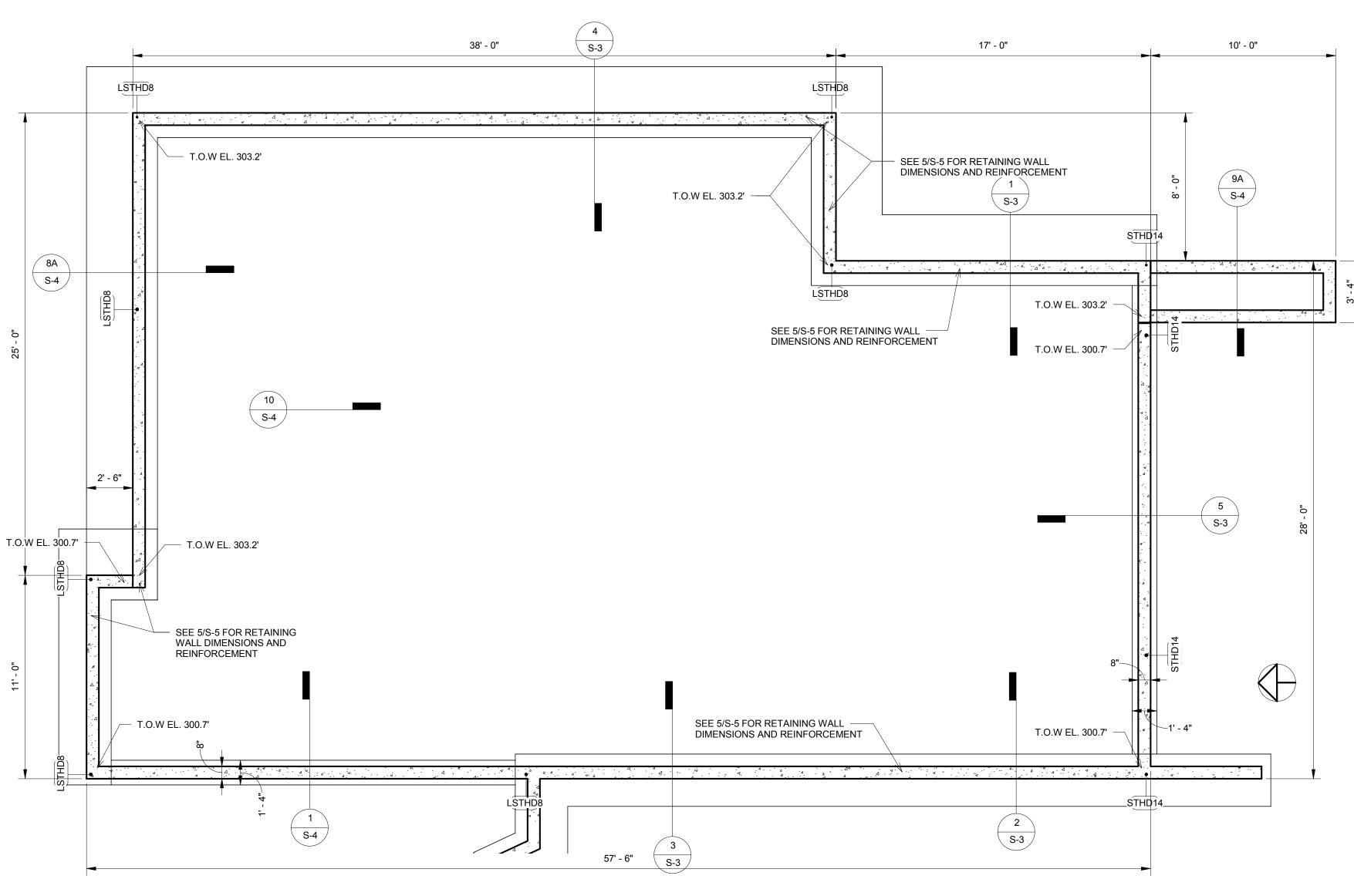
ENGINEER BB

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NO. DATE DESCRIPTION

GENERAL NOTES SPECIFICATIONS

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1 FOUNDATION PLAN
1/4" = 1'-0"



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IMPORTANT NOTES FOR CONTRACTOR:

CONTRACTOR SHALL REVIEW STRUCTURAL DRAWINGS AND FIELD VERIFY ALL RELATED EXISTING FRAMING & DIMENSIONS PRIOR TO ANY FIELD WORK. NOTIFY THE ENGINEER/OWNER ANY DISCREPANCIES FOUND IN THE FIELD. STRUCTURAL DRAWINGS MAY NOT CORRECTLY REFLECT ALL EXISTING FRAMING DUE TO LIMITED ACCESS TO THE SITE AND EXISTING DRAWINGS.

CONTRACTOR SHALL FIELD VERIFY AND NOTIFY THE ENGINEER/OWNER OF EXISTING MECHANICAL DUCTS, PLUMBING PIPES, ELECTRICAL WIRES THAT MAY INTERFERE WITH STRUCTURAL WORKS FOR COST CONSIDERATIONS PRIOR TO ANY FIELD WORK.

IMPORTANT FOUNDATION AND FRAMING NOTES:

1. ALL FOOTINGS SHALL BEAR ON SUITABLE SOIL SUCH AS MIN. OF MEDIUM DENSE NATIVE SOIL OR COMPACTED STRUCTURAL FILL (NO SOFT OR ORGANIC MATERIALS). GEOTECHNICAL ENGINEER MAY BE REQUIRED TO ASSESS EXISTING SOIL CONDITIONS.

2. FOR FRAMING LUMBER TYPES AND GRADES, AND CONCRETE MIX REQUIREMENTS PLEASE SEE S-0
3. FOR PLYWOOD/OSB SHEARWALL SCHEDULE, PLEASE SEE S-5

4. FOR COMMON HEADER FRAMING DETAIL AND HEADER SIZE, SEE S-5
5. PROVIDE (2) 2X6 OR (3) 2X4 STUD POSTS AT EACH END OF BEAMS, UNLESS NOTED OTHERWISE ON PLAN
6. SLAB ON GRADE SHALL BE 4" CONCRETE SLAB WITH #3 AT 18" EACH WAY

(MID-DEPTH) ON 6" COMPACTED CRUSHED ROCK. PROVIDE 1" SAWCUT JOINT AT 15 FT MAX. SPACING (EACH WAY)
7. FLOOR SHEATHING SHALL BE 3/4" PLYWOOD OR OSB WITH 10d AT 6" NAILING AT

EDGES AND AT 12" AT FIELD

8. ROOF SHEATHING SHALL BE 1/2" APA PLYWOOD WITH 16d (3.5" LONG) AT 6" NAILING AT BEAMS AND 10d (2" LONG) AT 12" FIELD NAILING

IMPORTANT NOTES ON TRUSS AND FLOOR FRAMING DESIGN AND SHOP DRAWING:

1. TRUSS FRAMING LAYOUT SHOWN IS GENERAL CONCEPT ONLY. CONTRACTOR/TRUSS SUPPLIER MUST SUBMIT TRUSS SHOP DRAWINGS INCLUDING TRUSS TEMPORARY/ PERMANENT BRACING PLANS FOR ENGINEER'S REVIEW 2. TRUSS FRAMING PROFILE/ LAYOUT SHOULD CONFORM TO BOTH STRUCTURAL AND ARCHITECTURAL DRAWINGS. ANY DEVIATIONS SHALL BE APPROVED BY ENGINEER/ ARCHITECT PRIOR TO TRUSS DESIGN WORK.

3. TRUSS DEFLECTION CRITERIAS:
FLOOR/DECK TOTAL LOAD = L/480 ROOF TOTAL LOAD = L/240
FLOOR/DECK LIVE LOAD = L/600 ROOF SNOW LOAD = L/300
** MAXIMUM TOTAL LOAD DEFLECTION SHOULD NOT EXCEED 1.0" IN ALL CASES
4. FLOOR/ROOF FRAMING LAYOUT AND CONNECTORS (SUCH AS LUMBER PACKAGE
BY SUPPLIERS) MUST BE SUBMITTED FOR ENGINEER'S REVIEW PRIOR TO

CONSTRUCTION
FRAMING SYMBOLS:

SS24 SIMPSON WSW STRONG POST

WALL (24" WIDE)

POST STOPS BELOW THIS FLOOR

SHEARWALL HOLDOWN

POST STARTS AT THIS FLOOR

LEGEND AND NOTES

1/4" = 1'-0"

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ENGINEER BB

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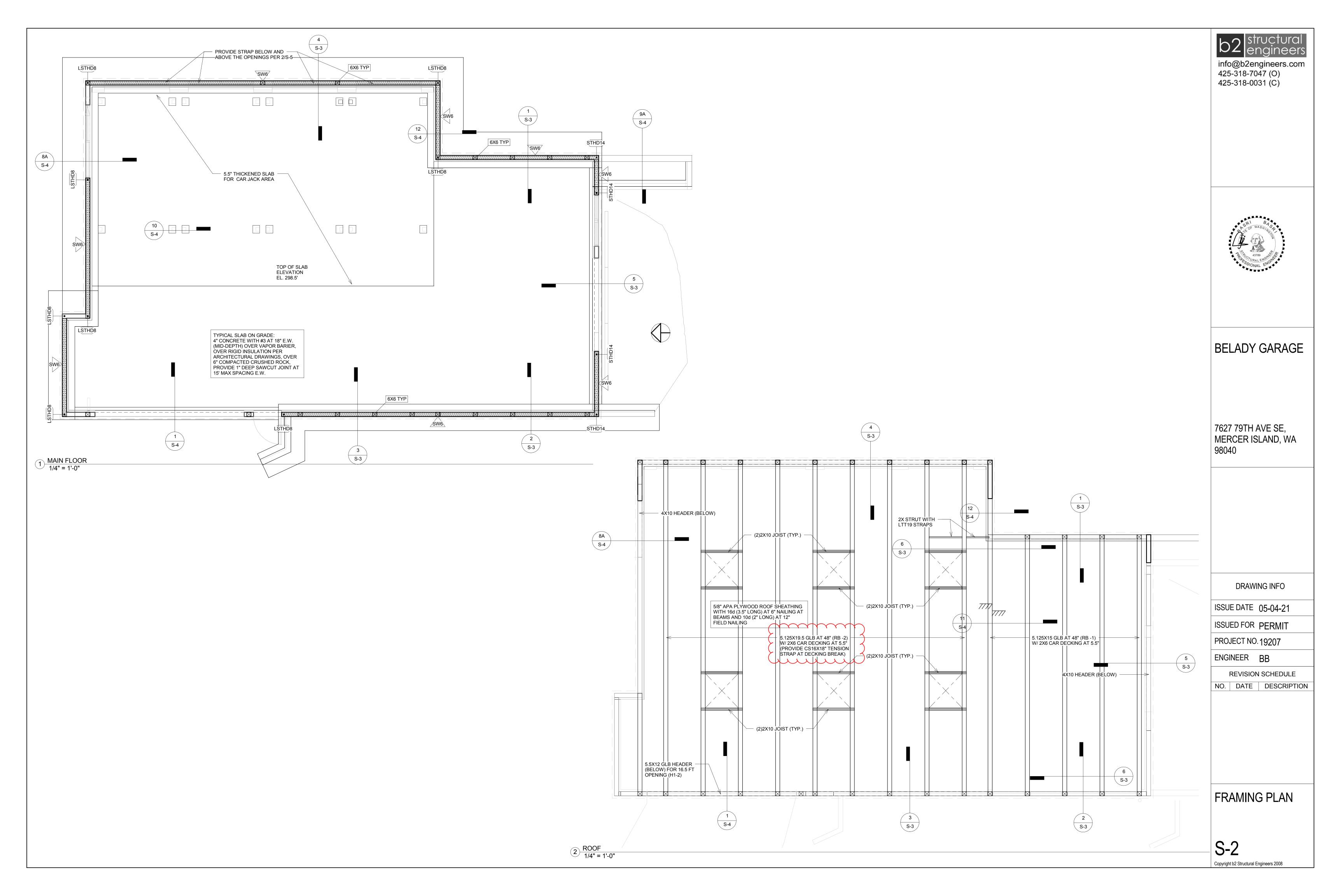
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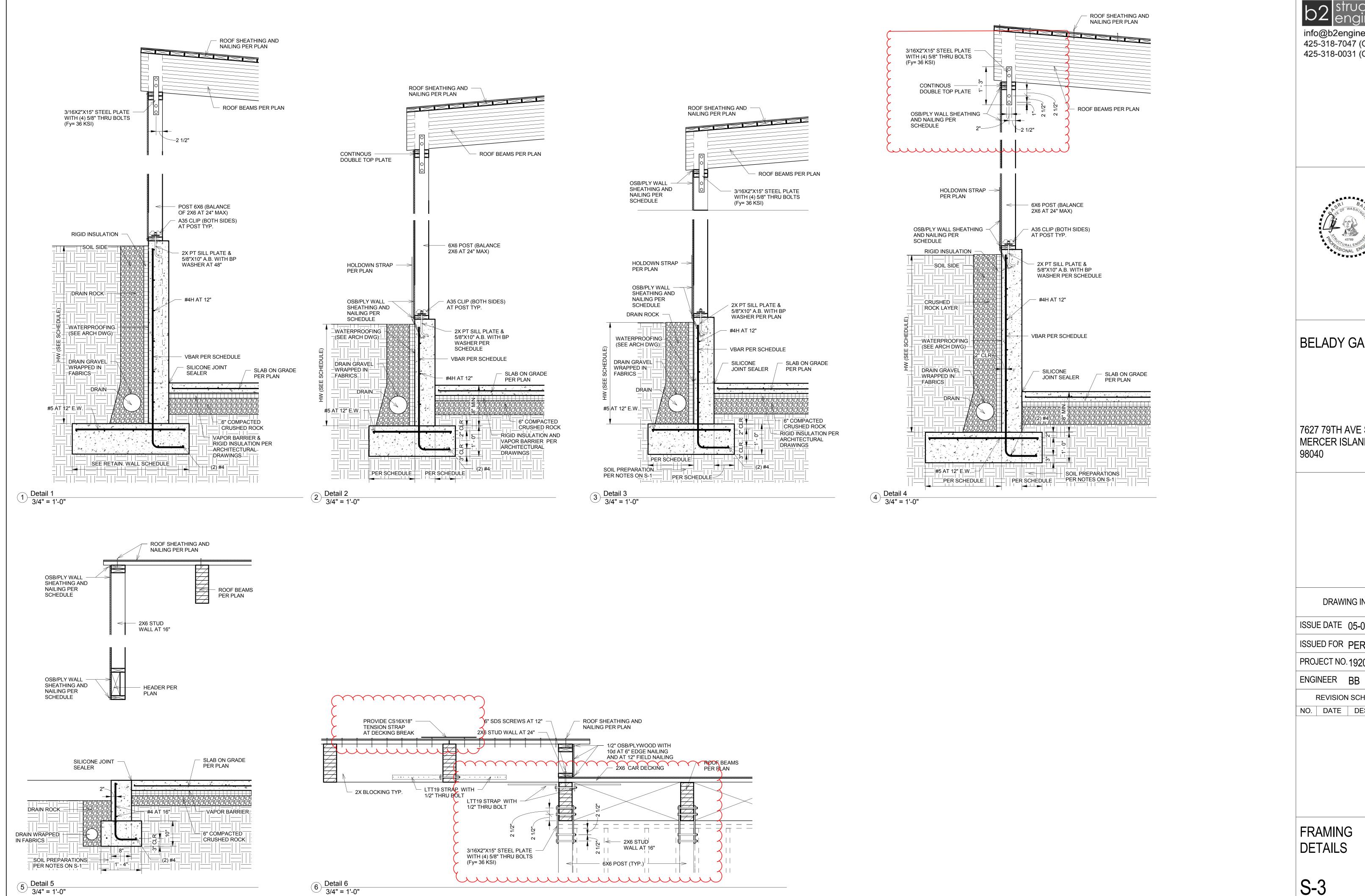
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FRAMING PLAN

S-1

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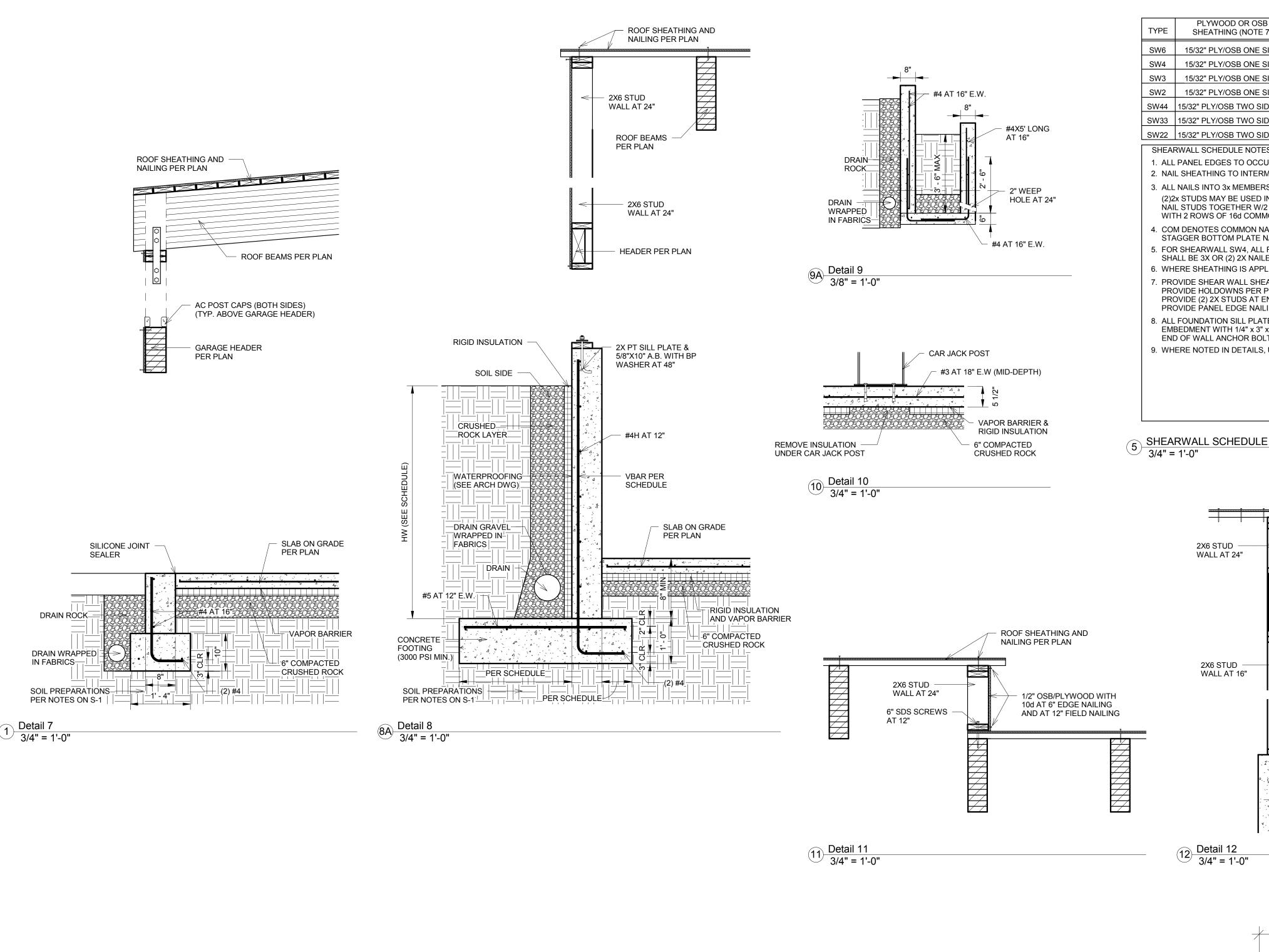
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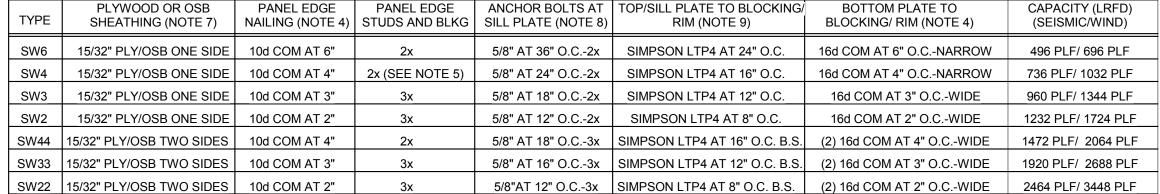
NO. DATE DESCRIPTION

FRAMING DETAILS

S-3

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SHEARWALL SCHEDULE NOTES:

2X6 STUD

WALL AT 24"

2X6 STUD

12 Detail 12 3/4" = 1'-0"

CONCRETE WALL (4000 PSI

VBAR

WALL AT 16"

- 1. ALL PANEL EDGES TO OCCUR OVER STUDS, PLATES, RIMS OR HORIZONTAL BLOCKING AT WALLS
- 2. NAIL SHEATHING TO INTERMEDIATE SUPPORTS/ FIELD NAILING 10d AT 12" O.C.
- 3. ALL NAILS INTO 3x MEMBERS SHALL BE STAGGERED (2)2x STUDS MAY BE USED IN LIEU OF 3x STUDS AT PANEL JOINTS.
- NAIL STUDS TOGETHER W/2 ROWS 16d COMMON AT 6" O.C. AT SINGLE SIDE SHEATHING AND NAIL WITH 2 ROWS OF 16d COMMON AT 3" O.C. AT DOUBLE SHEATHED WALLS.
- 4. COM DENOTES COMMON NAILS. MIN. NAIL PENETRATION INTO PLATE, RIM OR BLOCKING SHALL BE 1 5/8". STAGGER BOTTOM PLATE NAILING
- 5. FOR SHEARWALL SW4, ALL FRAMING MEMBERS RECEIVING EDGE NAILINGS FROM ABUTTING PANELS SHALL BE 3X OR (2) 2X NAILED TOGETHER WITH 16d AT 6" 6. WHERE SHEATHING IS APPLIED TO BOTH SIDES OF WALL, OFFSET PANEL EDGES TO FALL ON DIFFERENT STUDS.
- ". PROVIDE SHEAR WALL SHEATHING AND NAILING FOR ENTIRE LENGTH OF WALLS NOTED ON PLAN.
- PROVIDE HOLDOWNS PER PLAN AT EACH END OF WALL, UNO. PROVIDE (2) 2X STUDS AT ENDS OF ALL SHEARWALL. FACE NAIL MULTIPLE STUDS WITH 16d AT 12" PROVIDE PANEL EDGE NAILING IN EACH HOLDOWN STUD AT END OF WALL.
- 8. ALL FOUNDATION SILL PLATES SHALL BE PT MEMBERS AND THE ANCHOR BOLTS SHALL HAVE MIN. OF 7" EMBEDMENT WITH 1/4" x 3" x 3" PLATE WASHER OR SIMPSON'S BP/ BPS PLATE.

ROOF SHEATHING AND NAILING PER PLAN

6" SDS SCREWS

CONTINOUS ROOF

- PLYWOOD/OSB SHEATHING

CRUSHED

#4 AT 12"

ROCK LAYER

DAMPPROOFING (SEE ARCH DWG)

DRAIN GRAVEL

WRAPPED IN

FABRICS

#5 AT 12" E.W.

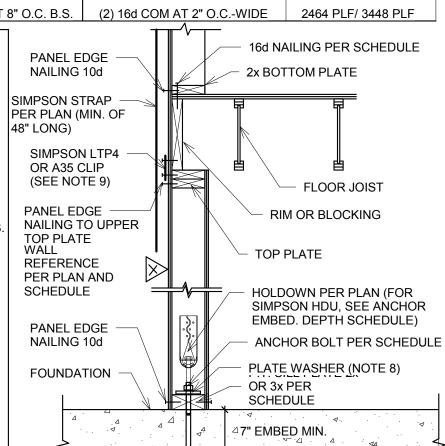
CONCRETE FOOTING (3000 PSI MIN.)

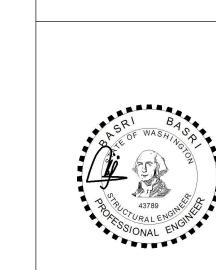
AND NAILING PER SHEARWALL SCHEDULE

2X PT SILL PLATE & 5/8"X10" A.B. WITH BP WASHER PER SCHEDULE

GLULAM BEAM

END OF WALL ANCHOR BOLTS SHALL BE LOCATED MAX 12" AND MIN 5" FROM END OF THE PLATE. 9. WHERE NOTED IN DETAILS, USE SIMPSON A35 IN LIEU OF LTP4 PLATES SPACE AT 2/3 OF LTP4 SPACING.





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NO. DATE DESCRIPTION

2. THE WALLS ARE <u>NOT</u> DESIGNED FOR WATER PRESSURE. IF RUNNING WATER IS ENCOUNTERED

STRUCTURAL ENGINEER Y 3. THE WALL CONCRETE STRENGTH SHALL BE

1. BACKFILL THE WALLS PRIOR TO FRAMING THE

DURING EXCAVATION, PLEASE NOTIFY THE

IMPORTANT NOTES:

FLOOR ABOVE

1'6" #4 AT 12" #4X3'X3' AT 12"

2'6" #4 AT 12" #5X4'X4' AT 12"

MIN. 2500 PSI.

4. THE FOOTING CONCRETE STRENGTH SHALL BE

5. DO NOT HESITATE TO CALL THE STRUCTURAL ENGINEER AT 425-296-2993 FOR ANY QUESTIONS

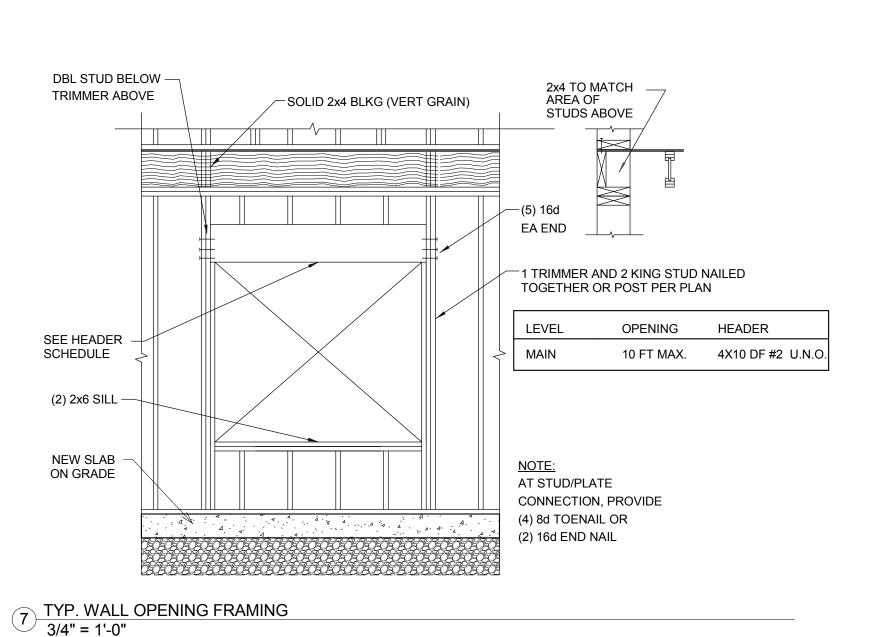
9 RETAINING WALL SCHEDULE
3/4" = 1'-0"

W2

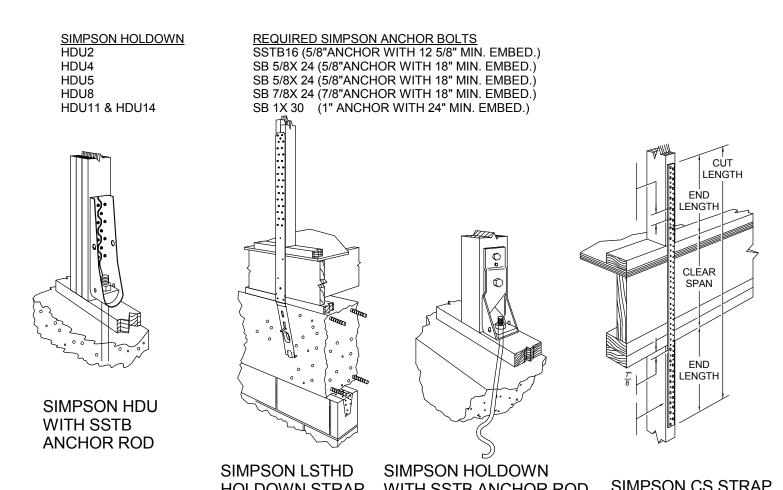
1' - 0"

FRAMING DETAILS

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3/4" = 1'-0"





HOLDOWN STRAP WITH SSTB ANCHOR ROD SIMPSON CS STRAP

HOLDOWN DETAILS & ANCHOR

8 SCHEDULE 3" = 1'-0"